

Roof systems: Technical data guide

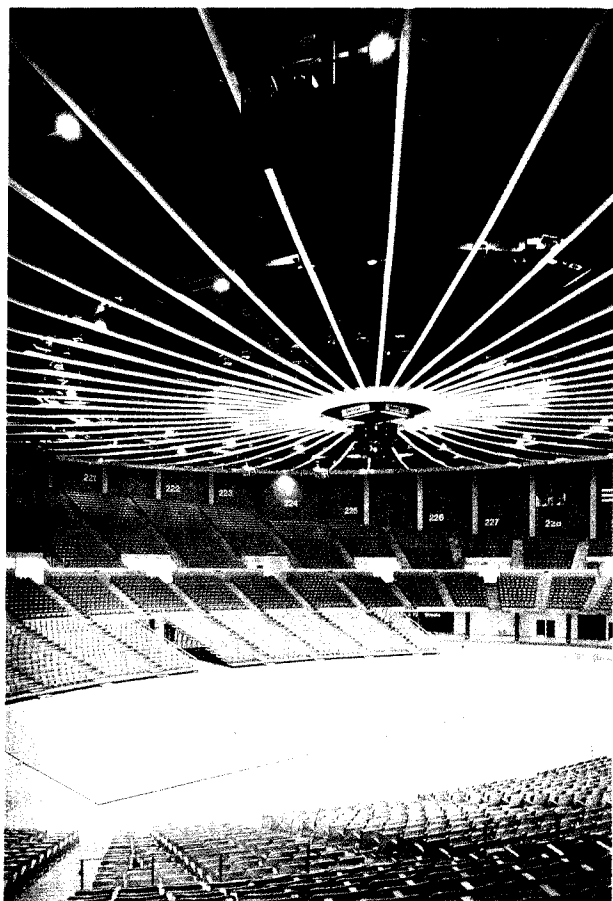
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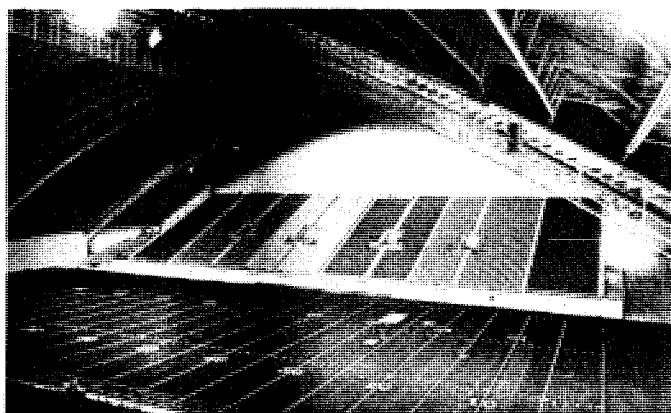


TABLE OF CONTENTS

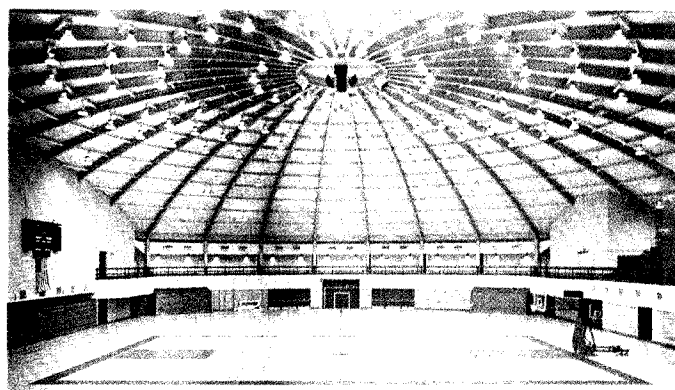
Applications	
Convention/Coliseum Facilities	3
Educational Institutions	4
Correctional Facilities	5
Power Plants and Special Purpose	6
Engineering Data — Structural	7
Properties, Load-Span Tables	
Section 3	8
Section 21	9
Section 12	10
Section 5-3.0	11
Section 5-4.5	12
Section 5-6.0	13
Section 5-7.5	14
DC/ADC-1.5	15
DC/ADC-3.0	16
DC/ADC-4.5	17
DC/ADC-6.0	18
DC/ADC-7.5	19
Cantilever Guide & Selection Table	20
Engineering Data — Acoustical	21
Engineering Data — Air Diffusion & Distribution	22-23
Typical Details — Sections 3, 21 & 12	24
Typical Details — DC/ADC	25
Typical Details — ADC Air Ceiling	26-27
Specifications — DC/ADC, ADC Air Ceiling	30-31
Photo Credits	32



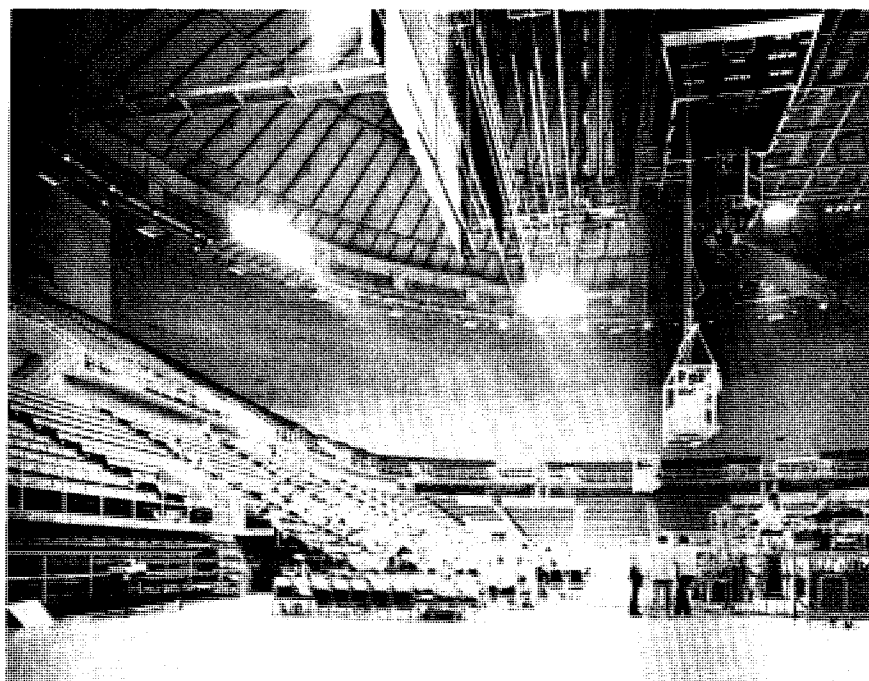
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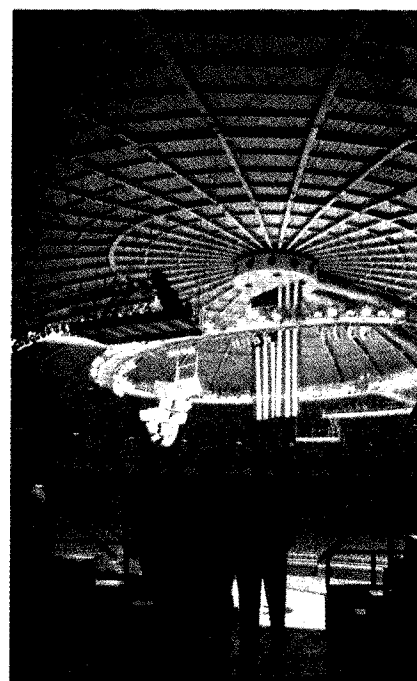
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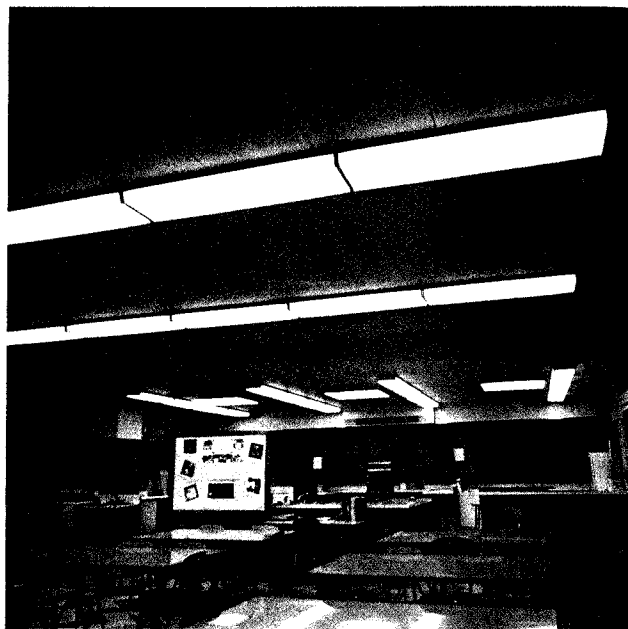


5

EDUCATIONAL INSTITUTIONS



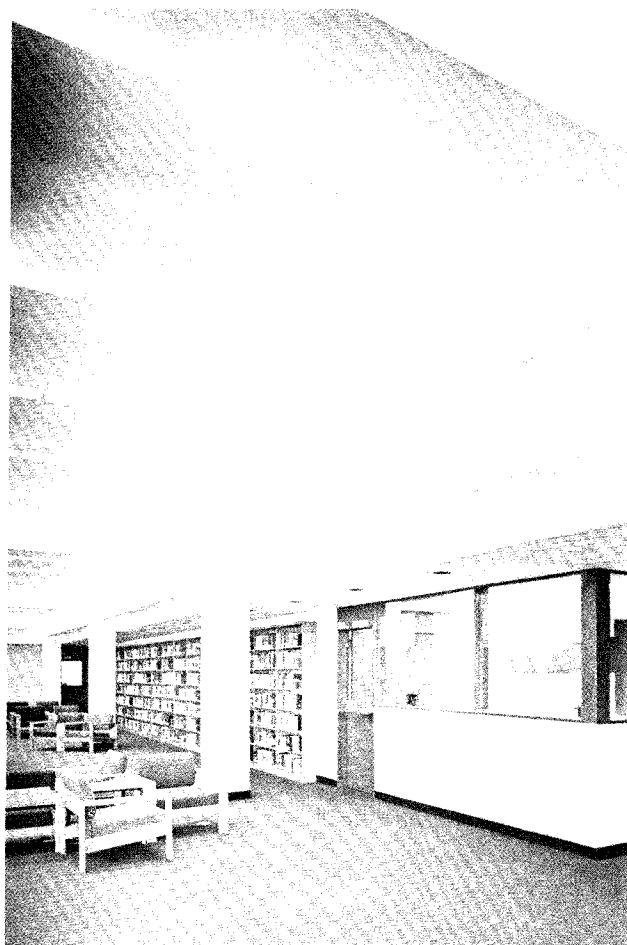
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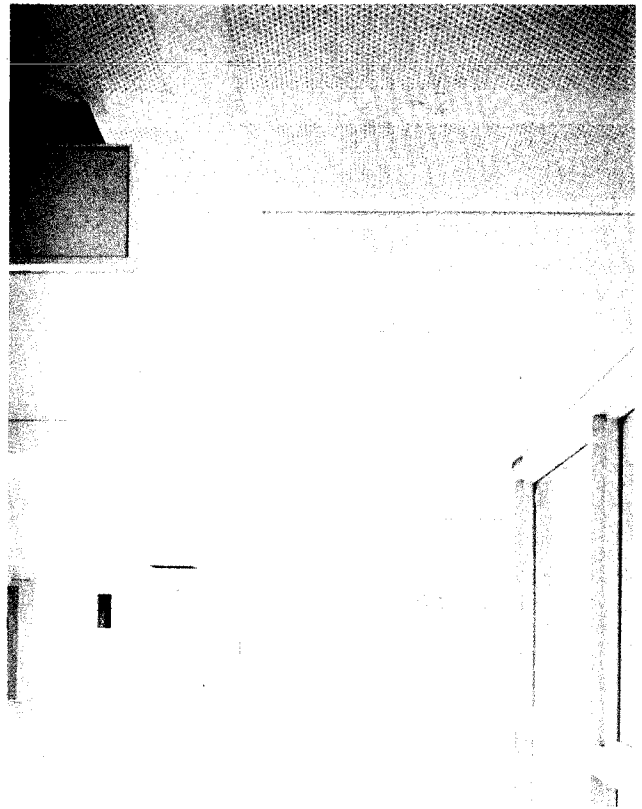
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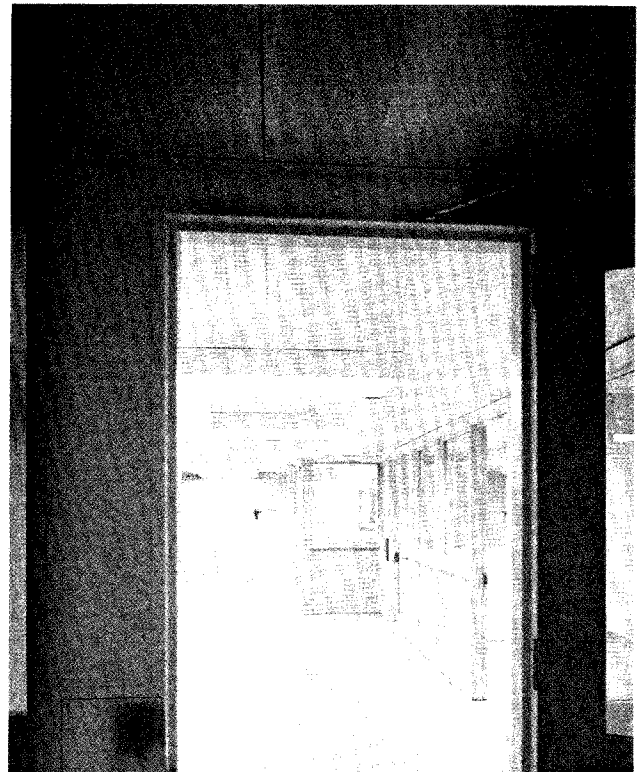
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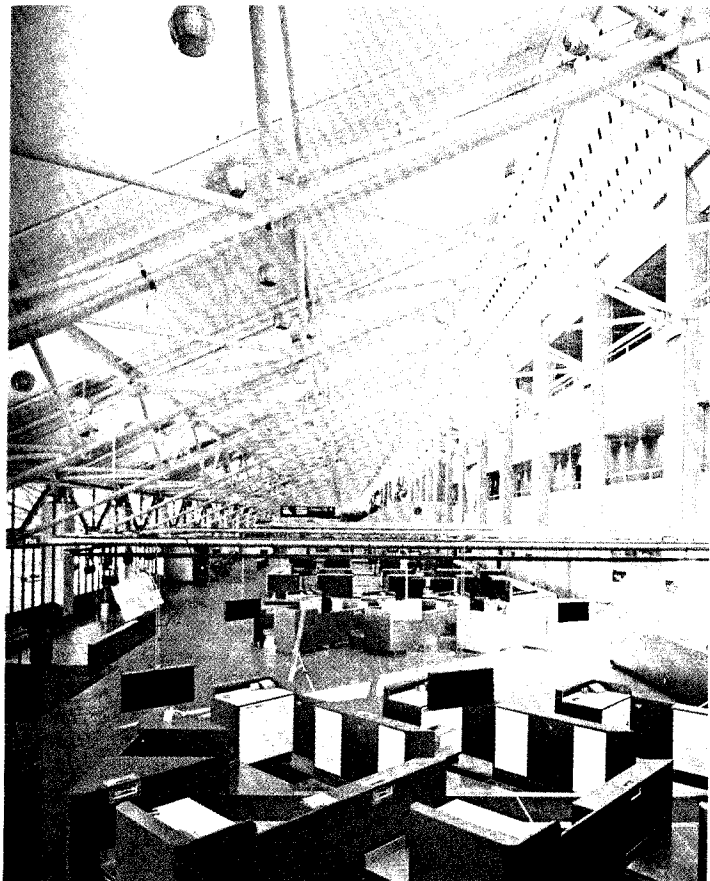


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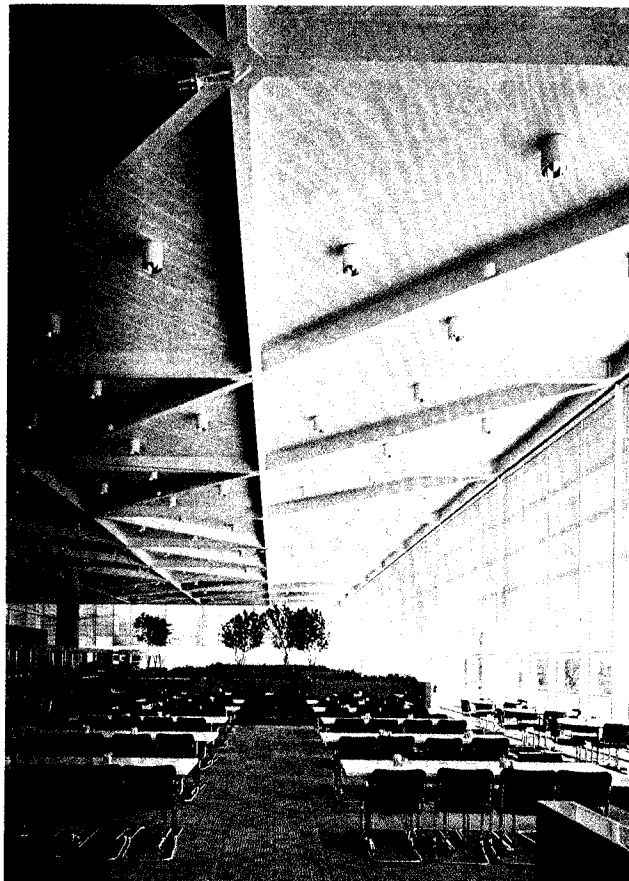


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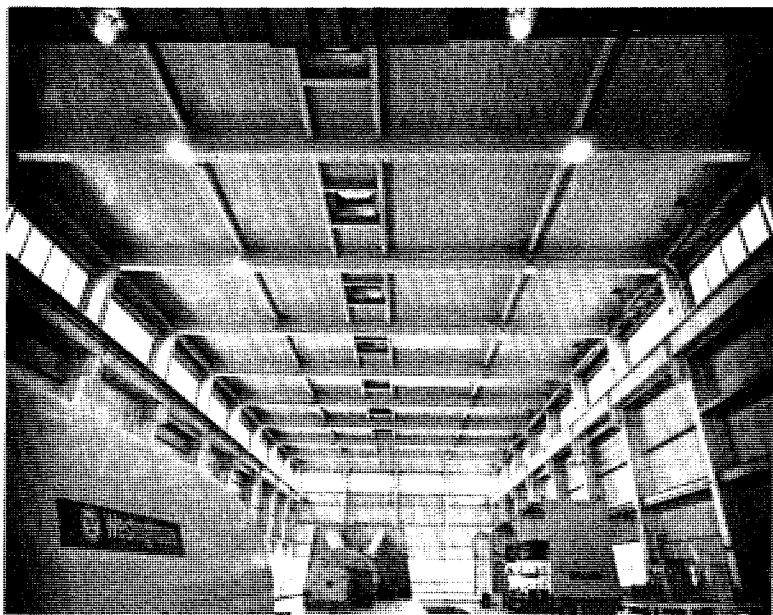
POWER PLANTS AND SPECIAL PURPOSE



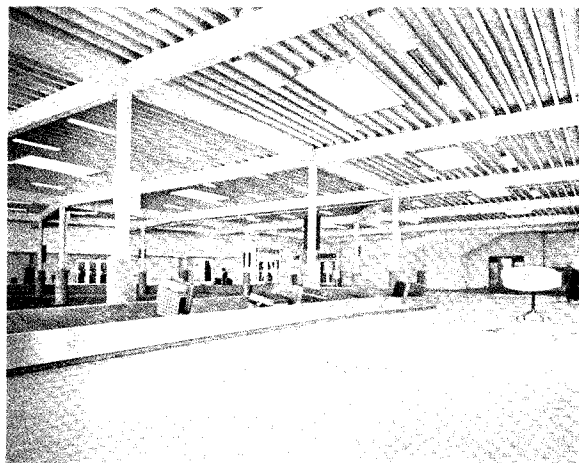
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16



17



ENGINEERING DATA - STRUCTURAL

SELECTING A ROBERTSON ROOF DECK SECTION

Designing a Robertson roof deck follows the procedure for any structural section. It is necessary to consider such items as the type of structure, type and degree of loading, span provisions, end bearing, and ceiling conditions, as well as humidity levels in the building.

The following data will enable the designer to select a steel roof deck. Most of the computation work normally required has been reduced to tabular form to simplify the selection of the appropriate deck unit.

1 — LOAD AND SPAN

A deck section must satisfy two basic requirements:

- (a) Strength — indicated by Section Modulus "S".
- (b) Deflection Resistance — indicated by Moment of Inertia "I".

The individual job and use of the deck will determine the dead and live load values. These are to be specified by the architect or designer and it is beneficial to have the values appear on the contract drawings. In addition, the architect will specify the type ceiling which will dictate the deflection criteria, normally 1/360 of the span for plastered ceilings and 1/240 of the span for unplastered ceilings. (Check local or state codes for accepted practice.)

2 — REACTION VALUES

End and intermediate reaction values are given in the Deck Section Property Table and are for 12" widths, the same as section property values. These values have been determined using web strength formulae promulgated by the American Iron and Steel Institute and are in full accordance with their accepted specification.

3 — LATERAL DIAPHRAGM DESIGN

Pertains to seismic, wind and bomb shock loadings. For further information see your local Robertson representative.

4 — LATERAL BRACING

Pertains to the stiffening of the compression flange of a beam. This should not be confused with Lateral Diaphragm Design.

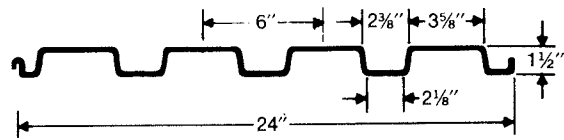
5 — HUMIDITY

Higher-than-normal interior relative humidity can require special consideration for the design and application of the steel deck. Contact your local Robertson representative with specific conditions.

DESIGN CONSIDERATIONS FOR SECTION PROPERTY AND LOAD-SPAN TABLES

1. All section properties have been determined by use of American Iron and Steel Institute's Specifications for Design of Light Gage Cold-Formed Steel Structural Members.
2. All properties are based on a 12" width of deck, although the units may be wider.
3. Recommended end bearing is 3 inches for roofs.
4. Allowable loads (all spans loaded) shown in tables have been rounded down to the nearest whole number. This number includes a provision of 7 psf for built-up roofing materials, plus the actual weight of the specific deck unit. None of the loads shown will produce a stress greater than 20,000 psi, or exceed end or intermediate bearing values as shown in the section property tables, or a deflection due to live load greater than:
 - a. Spans up to and including 20' center to center limited by a maximum deflection of $L/240$.
 - b. Spans over 21' center to center limited by a maximum deflection of 1".
 - c. Spans over 30' to be limited by the criteria of a maximum deflection of $L/360$.
5. End and intermediate reaction values vary with the length of bearing. For bearing lengths less than those shown in the catalog, consult AISC specifications or your local Robertson representative.
6. A moment coefficient of 1/10 has been used for 3 or more spans and 1/8 for two spans and simple spans.
7. A deflection coefficient of 2.65/384 has been used for 3 spans, and 2.08/384 for 2 spans and 5/384 for simple spans.
8. For cellular decks utilized in the inverted (flat-plate-up) position contact your Robertson representative.
9. Damage to the steel deck profile during the construction phase can significantly reduce its load carrying capacity. Deck erectors should exercise appropriate care to insure that any damaged deck is suitably strengthened or replaced. This is especially critical for decking placed on simple spans, since premature buckling caused by damaged deck can create a danger to workmen. For deck jobs where minor damage would be detrimental to underside appearance in the finished structure, suitably heavier deck gauges should be considered.
10. Roofs subject to water ponding and other similar phenomena (such as torrential rains) may need to be checked for load capacity. Because of the many factors involved, this responsibility rests with the structural designer.

SECTION 3



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 3	3-22	1.8	1.530	0.18	0.20	0.22	589	1687
	3-20	2.2	1.536	0.23	0.27	0.27	950	2303
	3-18	2.9	1.548	0.34	0.40	0.38	1841	3719
	3-16	3.5	1.560	0.44	0.51	0.48	2972	5407
	3-14	4.4	1.575	0.56	0.63	0.59	4697	7863
	3-12	5.9	1.605	0.76	0.88	0.88	9231	13996

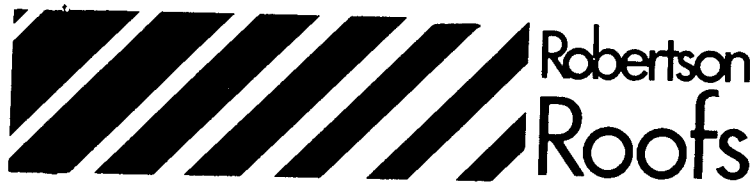
*End Bearing = 3" Intermediate Bearing = 4"

LOAD-SPAN TABLES

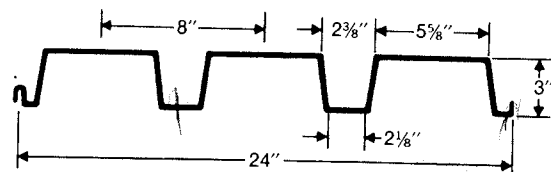
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET													
SECTION 3 MAX. LENGTH 40'-0"	22	1½"	SIMPLE DOUBLE TRIPLE	5.0	5.5	6.0	6.5	7.0	7.5	8.0	8.5	9.0					
				103	79	63											
				116	96	81	69	59	51	45	40	36					
				146	120	101	86	73	61	52	45	39					
	20	1½"	SIMPLE DOUBLE TRIPLE	6.0	6.5	7.0	7.5	8.0	8.5	9.0	9.5	10.0					
				78	64	53											
				99	86	74	64	56	50	44	40	36					
				126	107	92	76	64	55	48	42	37					
	18	1½"	SIMPLE DOUBLE TRIPLE	6.0	6.5	7.0	7.5	8.0	8.5	9.0	9.5	10.0	10.5	11.0	11.5	12.0	
				112	90	74	62	53	45	40	35	31					
				140	119	103	90	79	70	62	56	50	45	41	38	35	
				175	149	129	108	91	77	67	58	51	46	41	37	34	
	16	1½"	SIMPLE DOUBLE TRIPLE	6.5	7.0	7.5	8.0	8.5	9.0	9.5	10.0	10.5	11.0	11.5	12.0		
				115	94	79	67	57	50	44	39	35	32				
				151	130	113	100	88	79	70	64	58	52	48	44		
				189	163	140	117	99	85	74	65	57	51	46	42		

Note: For Factory Mutual Insured roofs, refer to FM 1-28 Bulletin for relevant design criteria.
Refer to page seven for additional design/erection considerations and limitations.



SECTION 21



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 21	21-22	2.1	3.030	0.67	0.39	0.47	403	1190
	21-20	2.6	3.036	0.85	0.50	0.58	663	1648
	21-18	3.5	3.048	1.26	0.76	0.79	1316	2706
	21-16	4.2	3.060	1.70	0.98	0.99	2154	3972
	21-14	5.2	3.075	2.26	1.26	1.23	3441	5820
	21-12	6.9	3.105	3.38	1.82	1.70	6841	10448

*End Bearing = 3" Intermediate Bearing = 4"

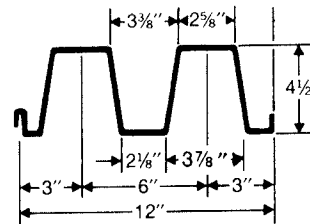
LOAD-SPAN TABLES

ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET											
				8.0	9.0	10.0	11.0	12.0							
SECTION 21 MAX. LENGTH 40'-0"	22	3"	SIMPLE DOUBLE TRIPLE	81											
				97	77	62	51								
				122	96	78	64	54							
	20	3"	SIMPLE DOUBLE TRIPLE	104	82	65									
				120	95	77	63	53	45						
				151	119	96	79	67	57						
	18	3"	SIMPLE DOUBLE TRIPLE	100	110	120	130	140	150	160	170	180			
				92	72	58	48	40	34						
				105	87	73	62	53	46	41	36	32			
				131	108	91	77								
	16	3"	SIMPLE DOUBLE TRIPLE	100	110	120	130	140	150	160	170	180	190	200	
				122	95	75	62	51	44	38	33				
				132	109	91	78	67	58	51	45	40	36	33	
				165	136	114	97								

Note: For Factory Mutual Insured roofs, refer to FM 1-28 Bulletin for relevant design criteria.
Refer to page seven for additional design/erection considerations and limitations.

SECTION 12



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 12	12-20	3.6	4.536	2.93	1.13	1.06	772	2281
	12-18	4.9	4.548	4.08	1.61	1.53	1577	3763
	12-16	5.9	4.560	5.19	2.11	2.04	2622	5518
	12-14	7.3	4.575	6.18	2.69	2.57	4239	8054
	12-12	10.0	4.605	8.59	3.43	3.60	8541	14338

*End Bearing = 3" Intermediate Bearing = 5"

LOAD-SPAN TABLES

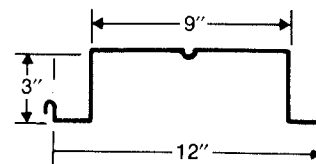
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET											
				10.0	11.0	12.0	13.0	14.0	15.0	16.0	17.0	18.0	19.0	20.0	
SECTION 12 MAX. LENGTH 40'-0"	20	4 1/2"	SIMPLE DOUBLE TRIPLE	150	124	104	88	76	66	57	49	43	38	34	
				140	117	98	83	72	62	55	48	43	39	35	
				176	145	122	104								
	18	4 1/2"	SIMPLE DOUBLE TRIPLE	12.0	13.0	14.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	
				149	127	109	91	77	66	57	50	45	39	34	
				142	121	104	90	79	70	63	56	51			
				177	151										
	16	4 1/2"	SIMPLE DOUBLE TRIPLE	13.0	14.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	23.0	
				166	136	113	96	82	71	62	55	47	41	37	
				160	138	120	106	94	83	75	67				
				200											

Refer to page seven for additional design/erection considerations and limitations.



SECTION 5-3.0



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 5-3.0	5-3.0-20	2.7	3.036	0.85	0.49	0.59	449	1289
	5-3.0-18	3.6	3.048	1.30	0.73	0.79	892	2085
	5-3.0-16	4.5	3.060	1.75	0.95	0.98	1459	3023
	5-3.0-14	5.4	3.075	2.26	1.20	1.22	2344	4402

*End Bearing = 3" Intermediate Bearing = 5"

LOAD-SPAN TABLES

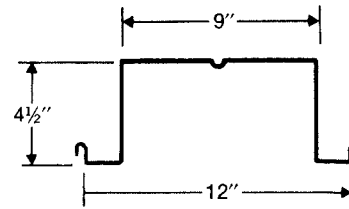
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET										
				8.0	9.0	10.0	11.0	12.0	13.0	14.0	15.0			
SECTION 5-3.0 MAX. LENGTH 40'-0"	20	3"	SIMPLE DOUBLE TRIPLE	101	80	64	51	41	35					
				123	97	78	65	54	46	40	34			
				153	121	98	81	68	57					
	18	3"	SIMPLE DOUBLE TRIPLE	153	120	95	74	59	49	41	35	31		
				164	129	105	87	72	62	53	46	41	36	32
				205	162	131	108	91	77					
	16	3"	SIMPLE DOUBLE TRIPLE	126	97	77	63	53	45	39	34			
				130	107	90	77	66	58	51	45	40	36	32
				163	134	113	96							

Refer to page seven for additional design/erection considerations and limitations.



SECTION 5-4.5



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 5-4.5	5-4.5-20	3.1	4.536	2.20	0.85	1.03	408	1367
	5-4.5-18	4.1	4.548	3.25	1.27	1.37	833	2227
	5-4.5-16	5.1	4.560	4.38	1.64	1.71	1384	3232
	5-4.5-14	6.5	4.575	5.68	2.08	2.12	2237	4669

*End Bearing = 3" Intermediate Bearing = 6"

LOAD-SPAN TABLES

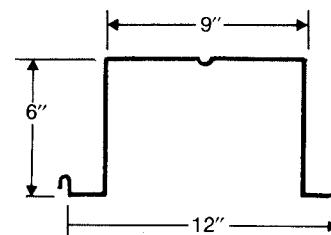
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET											
				10.0	11.0	12.0	13.0	14.0	15.0	16.0	17.0	18.0	19.0	20.0	
SECTION 5-4.5 MAX. LENGTH 40'-0"	20	4½"	SIMPLE	82	74	68	63	52	42	35					
			DOUBLE	109	99	91	81	70	61	53	47	42	38	34	
			TRIPLE	124	113	103	95								
	18	4½"	SIMPLE	166	139	118	100	86	74	63	54	47	42	37	33
			DOUBLE	178	150	126	100	93	81	71	63	56	50	45	
			TRIPLE	202	184	158	135								
	16	4½"	SIMPLE	151	129	111	96	82	70	61	53	48	41	36	
			DOUBLE	158	134	116	101	88	78	70	63	56			
			TRIPLE	197	168										

Refer to page seven for additional design/erection considerations and limitations.



SECTION 5-6.0



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 5-6.0	5-6.0-18	4.6	6.048	6.30	1.88	2.04	775	2117
	5-6.0-16	5.7	6.060	8.47	2.43	2.54	1310	3108
	5-6.0-14	7.2	6.075	10.99	3.09	3.16	2144	4528

* End Bearing = 3" Intermediate Bearing = 6"

LOAD-SPAN TABLES

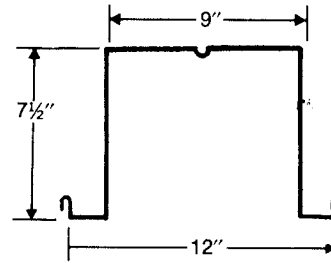
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET									
				16.0	17.0	18.0	19.0	20.0	21.0	22.0	23.0	24.0	25.0
SECTION 5-6.0 MAX. LENGTH 40'-0"	18	6"	SIMPLE DOUBLE	97	86	77	69	62	54	46	41	36	32
				106	94	83	75	67					
	16	6"	SIMPLE DOUBLE	126	111	99	89	80	69	60	52	46	41
				132	117	104	93	84					

Refer to page seven for additional design/erection considerations and limitations.



SECTION 5-7.5



PROPERTIES

DECK DESIGNATION	SECTION AND GAUGE	ACTUAL WT./SQ. FOOT POUNDS	OVER-ALL DEPTH in.	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	ALLOWABLE	
							END * REACTION lbs./ft.	INTER- * MEDIATE REACTION lbs./ft.
SECTION 5-7.5	5-7.5-18	5.1	7.548	10.59	2.56	2.79	716	2332
	5-7.5-16	6.4	7.560	14.21	3.31	3.48	1236	3428
	5-7.5-14	7.9	7.575	18.44	4.22	4.33	2051	4981
	5-7.5-13	9.4	7.590	22.36	5.11	5.19	3059	6757
	5-7.5-12	10.9	7.605	26.35	6.03	6.03	4246	8729

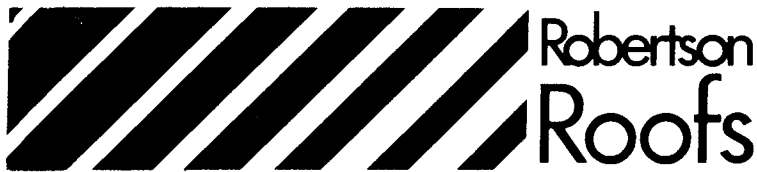
* End Bearing = 3" Intermediate Bearing = 7 1/2"

LOAD-SPAN TABLES

ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT.

DECK SECTION	GAUGE	RIB DEPTH	TYPE OF SPAN	PURLIN SPACING IN FEET											
				16.0	17.0	18.0	19.0	20.0	21.0	22.0	23.0	24.0	25.0		
SECTION 5-7.5 MAX. LENGTH 40'-0"	18	7 1/2"	SIMPLE DOUBLE	89	84	79	75	72	68	65	61	53	47		
				117	110	104	98	92							
	16	7 1/2"	SIMPLE DOUBLE	20.0	21.0	22.0	23.0	24.0	25.0	26.0	27.0	28.0	29.0	30.0	31.0
				110	100	91	80	69	61	54	48	43	39	36	33

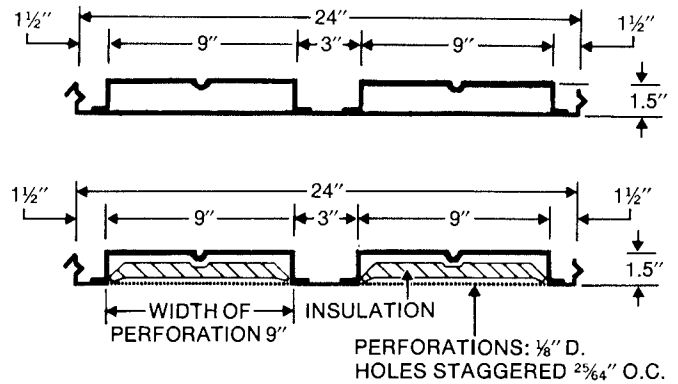
Refer to page seven for additional design/erection considerations and limitations.



DC-1.5

ADC-1.5

PROPERTIES



SECTION AND GAUGE	DC-ACTUAL WT./SQ. FOOT POUNDS ¹	OVER-ALL DEPTH in.	DC DECK			ADC DECK			ALLOWABLE INTER-MEDIATE REACTION	
			MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	END* REACTION lbs./ft.	INTER-MEDIATE REACTION lbs./ft.
1.5-20/18	4.0	1.584	0.36	0.26	0.48	0.34	0.25	0.44	490	1188
1.5-18/18	4.6	1.596	0.52	0.43	0.53	0.50	0.42	0.49	950	1919
1.5-18/16	5.1	1.608	0.56	0.43	0.64	0.54	0.43	0.60	950	1919
1.5-16/18	5.2	1.608	0.67	0.63	0.58	0.65	0.62	0.53	1533	2790
1.5-16/16	5.8	1.620	0.73	0.64	0.72	0.70	0.63	0.65	1533	2790
1.5-16/14	6.4	1.635	0.79	0.65	0.81	0.76	0.65	0.79	1533	2790
1.5-14/16	6.5	1.635	0.91	0.89	0.78	0.87	0.88	0.71	2423	4057
1.5-14/14	7.2	1.649	0.99	0.91	0.97	0.95	0.90	0.85	2423	4057
1.5-13/16	7.3	1.650	1.06	1.13	.084	1.01	1.12	0.77	3505	5541

¹ADC weights are approximately 4% less.

*End Bearing = 3" Intermediate Bearing = 4"

LOAD-SPAN TABLES

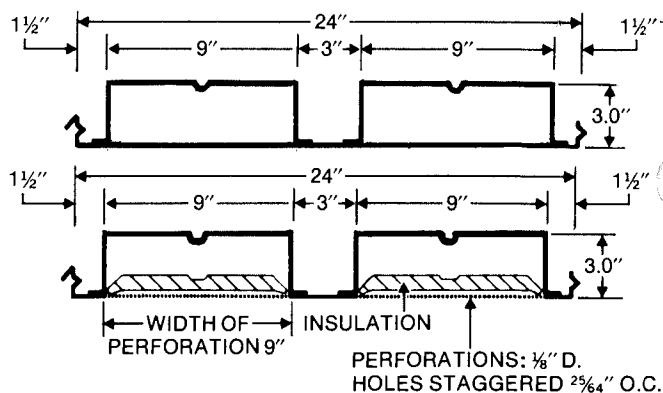
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

GAUGE	RIB DEPTH	TYPE OF SPAN	DC-1.5							ADC-1.5						
			PURLIN SPACING IN FEET							PURLIN SPACING IN FEET						
20/18	1 1/2"	SIMPLE	7.0	7.5	8.0	8.5	9.0	9.5	10.0	7.0	7.5	8.0	8.5	9.0	9.5	10.0
			69	60	53	47	42	37	34	69	60	52	46	41	37	33
		DOUBLE	130	114	100	88	78	70	63	119	104	91	81	72	64	58
			140	116	97	83	72	62	55	122	99	92	79	68	58	52
18/18	1 1/2"	SIMPLE	7.0	7.5	8.0	8.5	9.0	9.5	10.0	7.0	7.5	8.0	8.5	9.0	9.5	10.0
			110	92	78	66	58	51	45	106	88	75	64	56	49	44
		DOUBLE	145	126	111	98	87	78	71	132	115	101	89	80	71	64
			180	157	137	116	99	86	75	165	144	126	112	96	83	73
18/16	1 1/2"	SIMPLE	8.0	8.5	9.0	9.5	10.0	10.5	11.0	8.0	8.5	9.0	9.5	10.0	10.5	11.0
			83	71	62	54	48	43	39	80	69	60	53	47	42	38
		DOUBLE	133	118	105	94	85	77	70	124	110	98	88	79	72	65
			147	124	107	92	81	71	64	142	120	103	89	78	69	62
16/18	1 1/2"	SIMPLE	8.5	9.0	9.5	10.0	10.5	11.0	11.5	8.5	9.0	9.5	10.0	10.5	11.0	11.5
			84	72	63	56	50	45	41	80	69	61	54	48	43	39
		DOUBLE	107	95	86	77	70	64	58	98	87	78	71	64	58	53
			134	119	107	95	84	75	67	123	109	98	89	80	72	64
16/16	1 1/2"	SIMPLE	9.0	9.5	10.0	10.5	11.0	11.5	12.0	9.0	9.5	10.0	10.5	11.0	11.5	12.0
			78	68	60	54	48	44	40	75	66	58	52	47	42	39
		DOUBLE	118	106	96	87	79	72	66	106	95	86	78	71	65	59
			137	118	103	91	81	72	65	131	114	99	87	77	69	62

For spans not shown on charts, please consult your Robertson sales representative. Refer to page seven for additional design/erection considerations and limitations.



DC-3.0
ADC-3.0



PROPERTIES

SECTION AND GAUGE	DC-ACTUAL WT./SQ. FOOT POUNDS ¹	OVER-ALL DEPTH in.	DC DECK			ADC DECK			ALLOWABLE	
			MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	END* REACTION lbs./ft.	INTER.* MEDIATE REACTION lbs./ft.
3.0-20/18	4.4	3.084	1.56	0.60	1.04	1.50	0.59	0.99	449	1289
3.0-18/18	5.1	3.096	2.22	0.97	1.35	2.14	0.96	1.11	892	2085
3.0-18/16	5.7	3.108	2.38	0.99	1.39	2.30	0.98	1.35	892	2085
3.0-16/18	5.8	3.108	2.85	1.42	1.33	2.74	1.40	1.24	1459	3023
3.0-16/16	6.4	3.120	3.09	1.44	1.62	2.96	1.43	1.47	1459	3023
3.0-16/14	7.1	3.135	3.33	1.46	1.76	3.20	1.45	1.72	1459	3023
3.0-14/16	7.3	3.135	3.80	1.98	1.77	3.64	1.96	1.62	2344	4402
3.0-14/14	8.0	3.149	4.13	2.01	2.15	3.96	2.00	1.94	2344	4402
3.0-13/16	8.2	3.150	4.41	2.50	1.92	4.22	2.48	1.77	3393	5946

¹ADC weights are approximately 4% less.

*End Bearing = 3" Intermediate Bearing = 5"

LOAD-SPAN TABLES

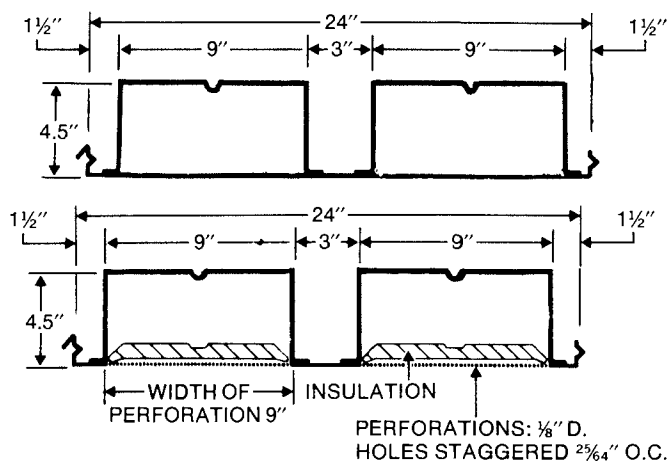
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

GAUGE	RIB DEPTH	TYPE OF SPAN	DC-3.0							ADC-3.0						
			PURLIN SPACING IN FEET							PURLIN SPACING IN FEET						
20/18	3"	SIMPLE DOUBLE TRIPLE	8.0	9.0	10.0	11.0	12.0	13.0	14.0	8.0	9.0	10.0	11.0	12.0	13.0	14.0
			112	98	79	65	55	47	40	112	97	79	65	54	46	40
			129	114	103	98	86	79	73	129	114	103	98	86	79	73
			146	130	117	106	97	90		146	130	117	106	97	90	
18/18	3"	SIMPLE DOUBLE TRIPLE	9.0	10.0	11.0	12.0	13.0	14.0	15.0	9.0	10.0	11.0	12.0	13.0	14.0	15.0
			159	129	106	89	76	65	55	158	127	105	89	75	62	53
			185	166	148	124	106	91	79	182	147	122	103	87	75	66
			210	189	172	156	124			210	189	152	128	109		
18/16	3"	SIMPLE DOUBLE TRIPLE	9.0	10.0	11.0	12.0	13.0	14.0	15.0	9.0	10.0	11.0	12.0	13.0	14.0	15.0
			162	131	108	90	77	67	58	160	130	107	90	77	66	56
			185	166	148	128	109	94	82	185	166	148	124	106	90	80
			210	189	172	158	133			210	189	172	156	129		
16/18	3"	SIMPLE DOUBLE TRIPLE	11.0	12.0	13.0	14.0	15.0	16.0	17.0	11.0	12.0	13.0	14.0	15.0	16.0	17.0
			153	121	97	80	68	58	50	147	116	94	77	65	56	49
			146	123	105	90	78	69	61	136	114	97	84	73	64	47
			183	154	131					170	143	121				
16/16	3"	SIMPLE DOUBLE TRIPLE	11.0	12.0	13.0	14.0	15.0	16.0	17.0	11.0	12.0	13.0	14.0	15.0	16.0	17.0
			158	130	105	87	73	62	54	157	125	101	83	70	60	52
			178	150	127	110	96	84	74	162	136	116	100	87	76	68
			223	187	159					203	170	145				

For spans not shown on charts, please consult your Robertson sales representative. Refer to page seven for additional design/erection considerations and limitations.



DC-4.5 ADC-4.5



PROPERTIES

SECTION AND GAUGE	DC-ACTUAL WT./SQ. FOOT POUNDS ¹	OVER-ALL DEPTH in.	DC DECK			ADC DECK			ALLOWABLE	
			MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	END* REACTION lbs./ft.	INTER-* MEDIATE REACTION lbs./ft.
4.5-20/18	4.6	4.584	3.80	1.03	1.66	3.67	1.02	1.62	408	1367
4.5-18/18	5.4	4.596	5.32	1.62	1.97	5.12	1.61	1.83	833	2227
4.5-18/16	5.9	4.608	5.71	1.65	2.23	5.50	1.63	2.19	833	2227
4.5-16/18	6.5	4.608	6.77	2.32	2.19	6.50	2.30	2.05	1384	3232
4.5-16/16	6.8	4.620	7.31	2.37	2.63	7.03	2.34	2.41	1384	3232
4.5-16/14	7.4	4.635	7.88	2.41	2.82	7.58	2.39	2.76	1384	3232
4.5-14/16	8.1	4.635	8.97	3.22	2.90	8.60	3.18	2.68	2237	4669
4.5-14/14	8.4	4.649	9.71	3.28	3.45	9.31	3.24	3.15	2237	4669
4.5-13/16	9.2	4.650	10.39	4.04	3.16	9.96	4.00	2.95	3282	6324

¹ADC weights are approximately 4% less.

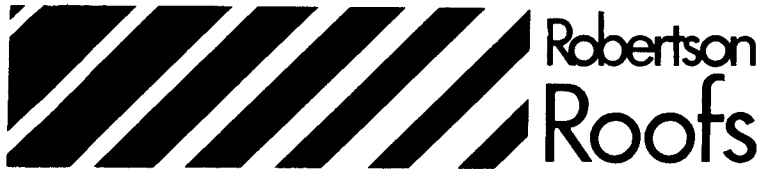
*End Bearing = 3" Intermediate Bearing = 6"

LOAD-SPAN TABLES

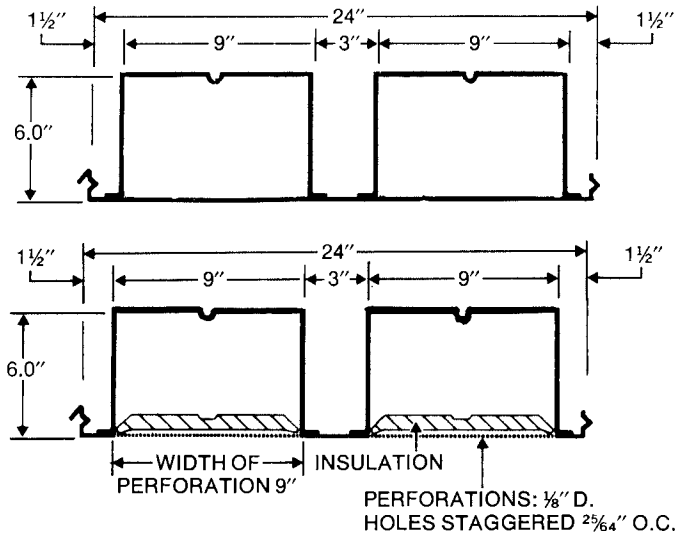
ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

GAUGE	RIB DEPTH	TYPE OF SPAN	DC-4.5								ADC-4.5							
			PURLIN SPACING IN FEET								PURLIN SPACING IN FEET							
20/18	4 1/2"	SIMPLE	12.0	13.0	14.0	15.0	16.0	17.0	18.0		12.0	13.0	14.0	15.0	16.0	17.0	18.0	
			68	63	58	54	51	47	42		68	63	58	54	51	46	41	
		DOUBLE	91	84	78	73	68	64	61		91	84	78	73	68	64	61	
			103	95							103	95						
18/18	4 1/2"	SIMPLE	12.0	13.0	14.0	15.0	16.0	17.0	18.0		12.0	13.0	14.0	15.0	16.0	17.0	18.0	
			150	127	110	96	84	74	66		148	126	109	95	83	74	66	
		DOUBLE	148	137	127	116	102	90	81		148	137	124	108	95	84	75	
			168	156							168	156						
18/16	4 1/2"	SIMPLE	12.0	13.0	14.0	15.0	16.0	17.0	18.0		12.0	13.0	14.0	15.0	16.0	17.0	18.0	
			152	130	112	97	85	76	67		151	128	111	96	85	75	67	
		DOUBLE	148	137	127	119	111	102	91		148	137	127	119	111	101	90	
			168	156							168	156						
16/18	4 1/2"	SIMPLE	14.0	15.0	16.0	17.0	18.0	19.0	20.0		14.0	15.0	16.0	17.0	18.0	19.0	20.0	
			158	137	120	103	89	78	69		156	136	117	100	86	75	66	
		DOUBLE	149	129	114	101	90	80	73		139	121	106	94	84	75	68	
16/16	4 1/2"	SIMPLE	15.0	16.0	17.0	18.0	19.0	20.0	21.0		15.0	16.0	17.0	18.0	19.0	20.0	21.0	
			140	123	109	96	83	73	63		138	122	107	92	80	71	61	
		DOUBLE	155	130	121	108	97	87			142	125	111	99	89	80		

For spans not shown on charts, please consult your Robertson sales representative.
Refer to page seven for additional design/erection considerations and limitations.



DC-6.0
ADC-6.0



PROPERTIES

SECTION AND GAUGE	DC-ACTUAL WT./SQ. FOOT POUNDS ¹	OVER-ALL DEPTH in.	DC DECK			ADC DECK			ALLOWABLE	
			MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	END* REACTION lbs./ft.	INTER-* MEDIATE REACTION lbs./ft.
6.0-18/18	6.0	6.096	10.00	2.37	2.81	9.63	2.34	2.63	775	2117
6.0-18/16	6.4	6.108	10.74	2.41	3.15	10.35	2.39	3.09	775	2117
6.0-16/18	7.1	6.108	12.65	3.34	3.15	12.16	3.30	2.97	1310	3108
6.0-16/16	7.3	6.120	13.64	3.41	3.73	13.11	3.37	3.45	1310	3108
6.0-16/14	8.0	6.135	14.70	3.48	3.98	14.13	3.44	3.89	1310	3108
6.0-14/16	8.9	6.135	16.67	4.58	4.15	16.01	4.53	3.87	2144	4528
6.0-14/14	9.1	6.149	18.01	4.68	4.86	17.29	4.63	4.49	2144	4528
6.0-13/16	10.0	6.150	19.30	5.72	4.56	18.53	5.65	4.28	3170	6168

¹ADC weights are approximately 4% less.

*End Bearing = 3" Intermediate Bearing = 6"

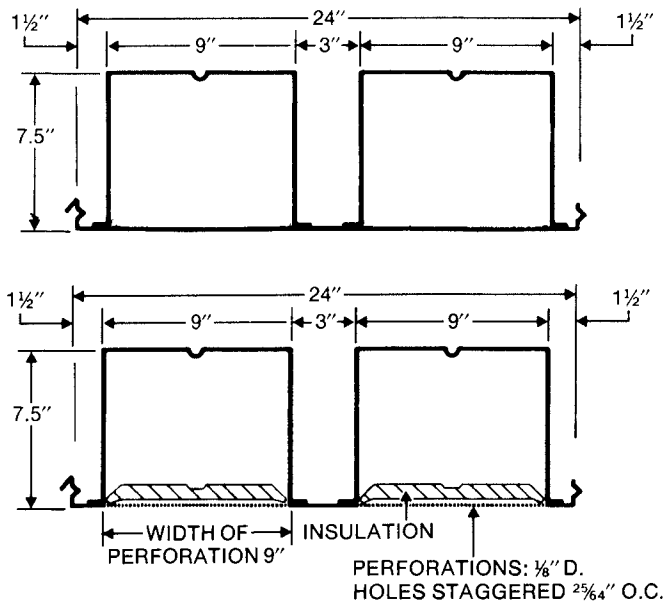
LOAD-SPAN TABLES

ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

GAUGE	RIB DEPTH	TYPE OF SPAN	DC-6.0								ADC-6.0							
			PURLIN SPACING IN FEET								PURLIN SPACING IN FEET							
18/18	6"	SIMPLE DOUBLE	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0
			103	97	91	86	80	77	71	65	103	97	91	86	81	77	70	64
			112	106	99	94	89	84			112	106	99	94	89	84		
18/16	6"	SIMPLE DOUBLE	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0
			103	97	91	86	81	77	71	66	103	97	91	86	81	77	71	65
			112	106	99	94	89	84			112	106	99	94	89	84		
16/18	6"	SIMPLE DOUBLE	19.0	20.0	21.0	22.0	23.0	24.0	25.0	26.0	19.0	20.0	21.0	22.0	23.0	24.0	25.0	26.0
			123	111	99	84	73	64	56	50	121	110	95	81	70	61	54	48
			111	105							109	98						
16/16	6"	SIMPLE DOUBLE	19.0	20.0	21.0	22.0	23.0	24.0	25.0	26.0	19.0	20.0	21.0	22.0	23.0	24.0	25.0	26.0
			125	113	103	90	78	68	60	53	124	112	101	87	75	66	58	51
			111	105							111	105						



DC-7.5
ADC-7.5



PROPERTIES

SECTION AND GAUGE	DC-ACTUAL WT./SQ. FOOT POUNDS ¹	OVER-ALL DEPTH in.	DC DECK			ADC DECK			ALLOWABLE	
			MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	MOM. OF INERTIA in. ⁴	SECTION MODULUS + MOMENT in. ³	SECTION MODULUS - MOMENT in. ³	END* REACTION lbs./ft.	INTER-* MEDIATE REACTION lbs./ft.
7.5-18/18	6.6	7.595	16.44	3.20	3.73	15.83	3.16	3.50	716	2332
7.5-18/16	6.9	7.608	17.66	3.27	4.14	17.01	3.23	4.06	716	2332
7.5-16/18	7.7	7.608	20.68	4.46	4.21	19.90	4.40	3.99	1236	3428
7.5-16/16	8.0	7.620	22.28	4.56	4.92	21.42	4.50	4.57	1236	3428
7.5-16/14	9.2	7.635	23.99	4.66	5.23	23.08	4.61	5.12	1236	3428
7.5-14/16	9.6	7.635	27.15	6.07	5.52	26.11	6.00	5.17	2051	4981
7.5-14/14	10.0	7.649	29.29	6.21	6.38	28.14	6.14	5.94	2051	4981
7.5-13/16	11.0	7.650	31.44	7.55	6.11	30.25	7.45	5.76	3059	6757

¹ADC weights are approximately 4% less.

*End Bearing = 3" Intermediate Bearing = 7 1/2"

LOAD-SPAN TABLES

ALLOWABLE UNIFORM TOTAL LOADS IN POUNDS PER SQUARE FOOT

GAUGE	RIB DEPTH	TYPE OF SPAN	DC-7.5									ADC-7.5								
			PURLIN SPACING IN FEET									PURLIN SPACING IN FEET								
18/18	7 1/2"	SIMPLE DOUBLE	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0
			95	89	84	79	75	71	68	65	95	89	84	79	75	71	68	65	95	89
			124	116	110	103	98	93			124	116	110	103	98	93			124	116
18/16	7 1/2"	SIMPLE DOUBLE	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0	17.0	18.0	19.0	20.0	21.0	22.0	15.0	16.0
			95	89	84	79	75	71	68	65	95	89	84	79	75	71	68	65	95	89
			124	116	110	103	98	93			124	116	110	103	98	93			124	116
16/18	7 1/2"	SIMPLE	22.0	23.0	24.0	25.0	26.0	27.0	28.0	29.0	22.0	23.0	24.0	25.0	26.0	27.0	28.0	29.0	22.0	23.0
			112	107	96	84	74	65	58	53	112	107	93	81	71	63	56	51	112	107
16/16	7 1/2"	SIMPLE	22.0	23.0	24.0	25.0	26.0	27.0	28.0	29.0	22.0	23.0	24.0	25.0	26.0	27.0	28.0	29.0	22.0	23.0
			112	107	103	88	78	69	62	56	112	107	99	86	76	67	60	54	112	107

For spans not shown on charts, please consult your Robertson sales representative.
Refer to page seven for additional design/erection considerations and limitations.

CANTILEVER GUIDE & SELECTION TABLE

FORMULA FOR DETERMINING REQUIRED SECTION PROPERTIES FOR CANTILEVER SELECTION

1. Anchor span governs when $L < 0.414A$

$$\text{Req'd. } S = 0.000075 \frac{W_T}{A^2} (A+L)^2 (A-L)^2$$

$$\text{Req'd. } I = 0.0000305 W_L A (5A^2 - 12L^2)$$

Note—Formula for req'd. I based on simplifying assumption that Δ_{\max} occurs at $A/2$

2. Cantilever governs when $L > 0.414A$

$$\text{Req'd. } S = 0.0003 W_T L^2$$

$$\text{Req'd. } I = 0.0000244 W_L (4L^2 A - A^3 + 3L^3)$$

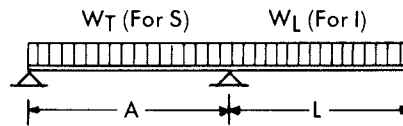
W_T = total load in psf

W_L = live load in psf

A = anchor span in feet

L = cantilever overhang in feet

In deriving the above formula, $f_b = 20000$ psi
and $E = 29,500,000$ psi maximum deflection:
for cantilever $\Delta_{\max} = L/120$ and for anchor span,
 $\Delta_{\max} = A/240$.

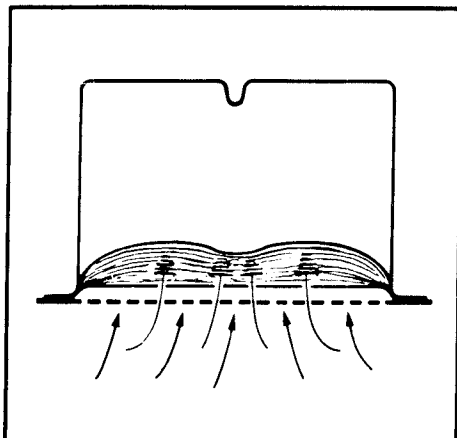


Total Load 45 psf
Live Load 30 psf
Deflection $L/120$ (or $A/240$)
 $L = 0.866 \times A$

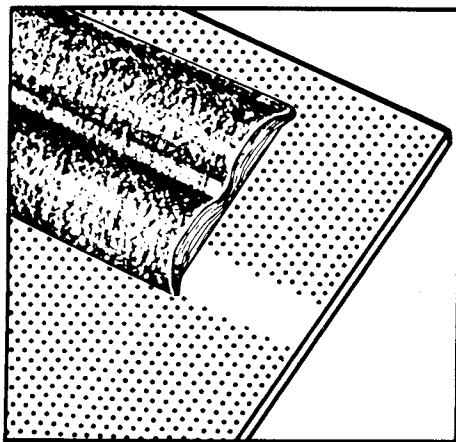
L (ft.)	REQUIRED		SELECTION TABLE • REQUIRED SECTION		
	S (in. ³)	I (in. ⁴)			
5	0.34	0.55	21-18 3-14	5-3.0-20	DC-3.0-20/18 DC-1.5-18/16
6	0.49	0.95	21-18 12-20	5-3.0-18	DC-3.0-20/18
7	0.66	1.50	12-20 21-16	5-4.5-20	DC-3.0-18/18
8	0.86	2.24	12-20 21-14	5-4.5-18	DC-3.0-18/16
9	1.09	3.19	12-18 21-12	5-4.5-18	DC-4.5-18/18 DC-3.0-16/14
10	1.35	4.38	12-16	5-4.5-16	DC-4.5-18/18 DC-3.0-13/16
11	1.63	5.83	12-14	5-6.0-18	DC-4.5-16/18
12	1.94	7.56	12-12	5-6.0-16	DC-6.0-18/18 DC-4.5-16/14 DC-4.5-14/16
13	2.28	9.62		5-7.5-18 5-6.0-14	DC-6.0-18/18 DC-4.5-14/14
14	2.65	12.01		5-7.5-16	DC-6.0-16/18
15	3.04	14.77		5-7.5-14	DC-7.5-18/18 DC-6.0-14/16

- For the listed sections, web crippling and weld strength are not critical for wind uplift forces less than 30 psf.

Sound absorption



After it penetrates the perforated metal surface, sound energy is absorbed by the glass fiber pad and the air space above. The use of "metal pan" acoustical ceilings is well known and is recognized as one of the more effective acoustic ceiling methods. The arch shape of the sound absorbing element insures its support above the perforations to permit the sound energy to penetrate the pad. This self-supporting shape holds the pad above the holes to prevent clogging during subsequent field painting operations.



Long Span Acoustical ADC units combine a perforated plate, which also becomes the finished ceiling, with an internal sound absorbing element. The plate is perforated with paths of $\frac{1}{8}$ " diameter holes, providing approximately 10% open area. A specially formed arched pad of extra fine glass fibers is provided as the sound absorbing medium. It is field installed above the perforated plate. The high degree of acoustical correction is readily seen in the adjoining table which summarized NRC values.

FREQUENCIES							
DECK	125	250	500	1000	2000	4000	NRC
COEFFICIENTS							
ADC-1.5	.24	.45	.65	.93	.75	.52	.70
ADC-3.0	.62	.61	.82	.83	.66	.60	.75
ADC-4.5	.37	.79	.98	.81	.70	.50	.80
ADC-6.0	.63	.94	.93	.74	.70	.48	.85
ADC-7.5	.67	1.09	.87	.67	.70	.47	.85

All of the above data, with exception of ADC-60, is from actual tests conducted at recognized acoustical laboratories following the provisions of ASTM C423-66 and using a number 4 mounting. All tests were conducted using the standard perforation pattern for ADC and standard thermally molded arched glass fiber insulation pads. Should values in excess of those shown be required, changes in the depth and density of the insulation can be made. Values for ADC-60 have been interpolated.

Sound transmission

The transmission of sound from one room to another can readily occur when partitions stop at the underside of a ceiling. Sound waves can enter the plenum and be transmitted to adjoining areas unless properly blocked.

In a suspended acoustical ceiling, it is generally costly and difficult to effect a sound barrier in the plenum above partitions. However, this is readily handled within the confines of the cells of Long Span units.

Directly over non-load-bearing partitions, sound barriers of formed glass fiber are inserted in the deck cells. Above load bearing partitions, specially formed glass fiber cells closures are inserted in the deck ends. Sound transmission from room to room is reduced to a level equal to or less than that of the walls.

ADC Deck provides concealed delivery of conditioned air

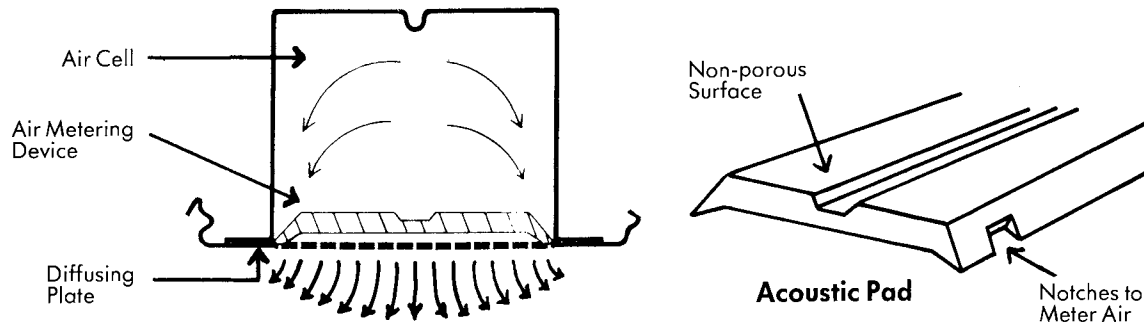
One additional function that can be obtained from the cellular floor or roof section is the transfer and diffusion of conditioned air into an interior space. The large cross section of the deck affords a generous "air duct," and the perforations provide the diffusing medium. The result is an air delivery method that eliminates branch duct work, and exposed grilles and provides for a uniform delivery of air across the entire width of the room. No hot or cold spots — no uncomfortable or drafty location. Utilizing the structural decking also reduces site assembly and connections and makes the "system" truly functional in nature.

ADC Deck as an Air Ceiling is simply a pressurized plenum that diffuses cool or warm air into a space spanned by the deck assembly above. The specially notched and coated acoustic pad is the "metering device" that transfers air from the upper distributing chamber into the lower diffusing chamber. The static pressure of

the mechanical system, the notch pattern and the continuous perforation pattern in the ceiling plate combine to deliver air to the space below in a most uniform fashion the entire span length. Cooling or heating loads for individual room requirements will determine the number of cells to be activated for air diffusion.

Air capacities are a direct function of the depth of the deck. Depth is usually governed by structural considerations. Adjustment of each deck gauges can also be helpful in providing an optimum of structural/air handling criteria.

Air capacities could have been calculated by standard ASHRAE formulae. However, because the air cell, metering device and perforations act in series, it was considered essential to run actual tests to check flow patterns, velocities and other aspects. Various lengths, different depths of deck, methods of air supply, location of closures and baffles were tested to arrive at the indicated design information summarized below.



AIR QUANTITY CAPACITIES

ADC DECK DEPTH	AREA OF EQUIVALENT ROUND	MAXIMUM CAPACITIES*
4.5"	5.5" (above metering pad)	100 cfm
6.0"	6.9" (above metering pad)	200 cfm
7.5"	8.0" (above metering pad)	320 cfm

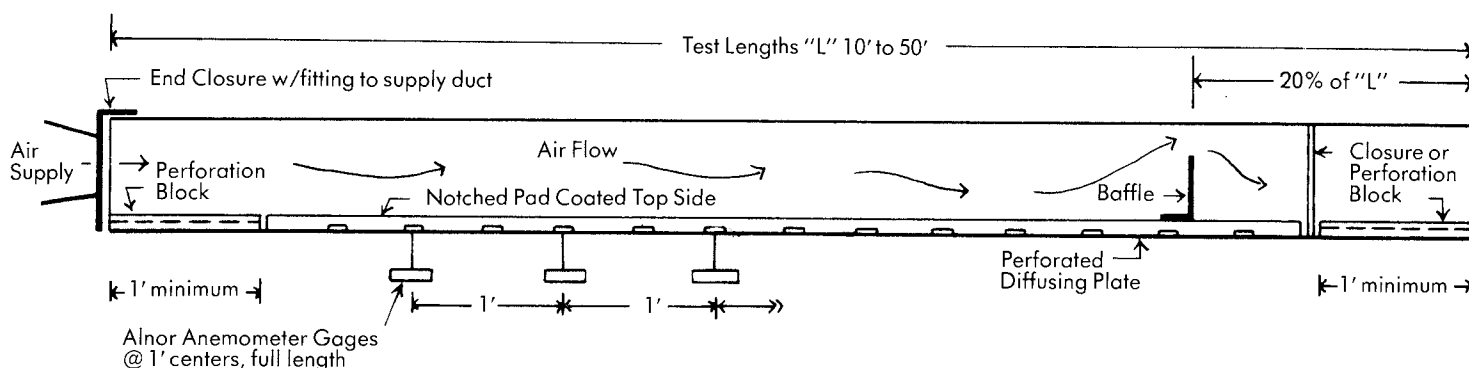
Design Notes:

1. Flow Range — Maintain between 3 and 10 cfm/lineal foot.
2. Place perforation blocks or closures to keep air diffusion a minimum of 1' from any wall.
3. When used in ceiling/roof construction — insulate voids to each side of ADC Air Cell.
4. At the indicated capacities* there is no detectable sound level of inlet air or diffused air.

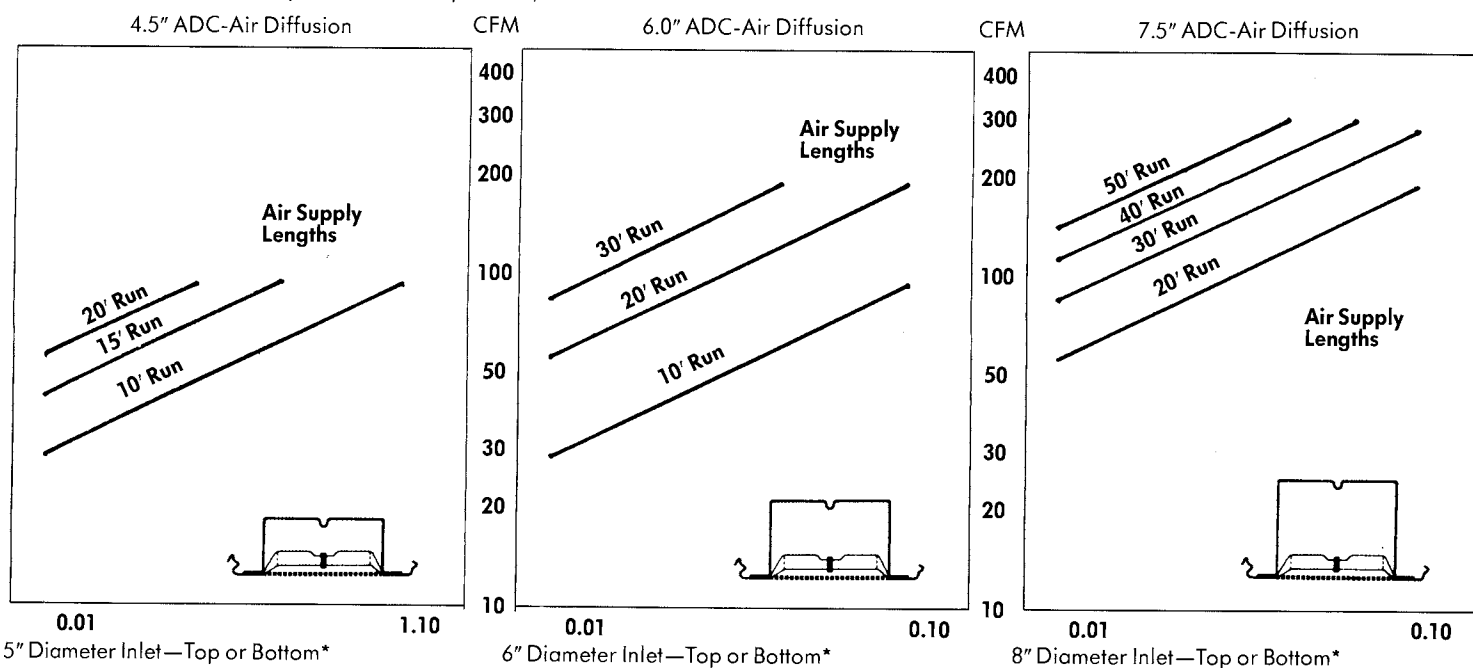
Test procedures and design value

The schematic drawing below shows the basic test assembly. Alnor Thermal Anemometers spaced 1' along the air cells were the instrumentation. Velocity readings and variations were taken as a direct measure of volume delivery. The total volume variation at 3

cfm per lineal foot, measured +1.5% to -2% along the length. At 10 cfm per lineal foot, volume variation measured +9.0% to -6.0% along the length. The most efficient location of the baffle detail was found to be constant for all lengths of air cell, at a position equal to 20% of the total length measured from the end opposite the delivery.



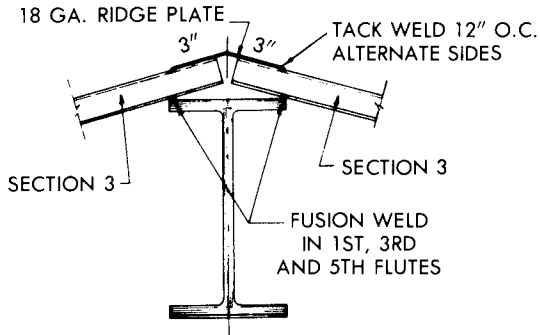
Friction loss charts—Required inlet static pressure, inches water.



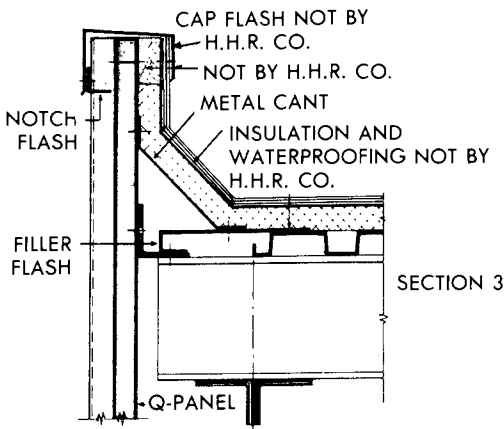
*Static pressure required for air supply through an end inlet is less than for a top or bottom inlet. Use equivalent rectangular area for size of end inlet fitting.

TYPICAL DETAILS - STRUCTURAL

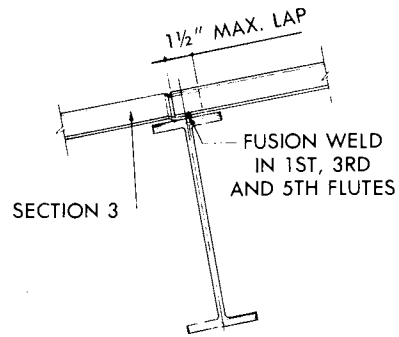
Sections 3, 21 and 12



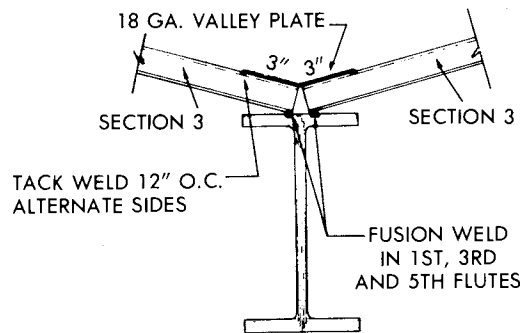
RIDGE DETAIL—ROOF



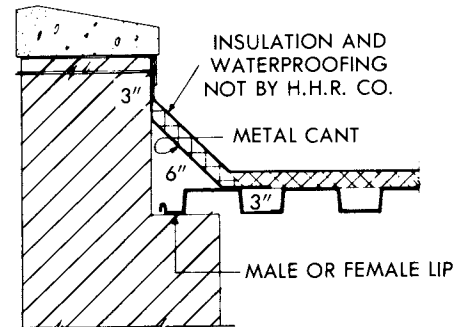
ROOF CELLS PARALLEL TO PARAPET WALL (Q-PANEL)



TYPICAL END JOINT DETAIL SHOWING LAP OVER ROOF PURLIN

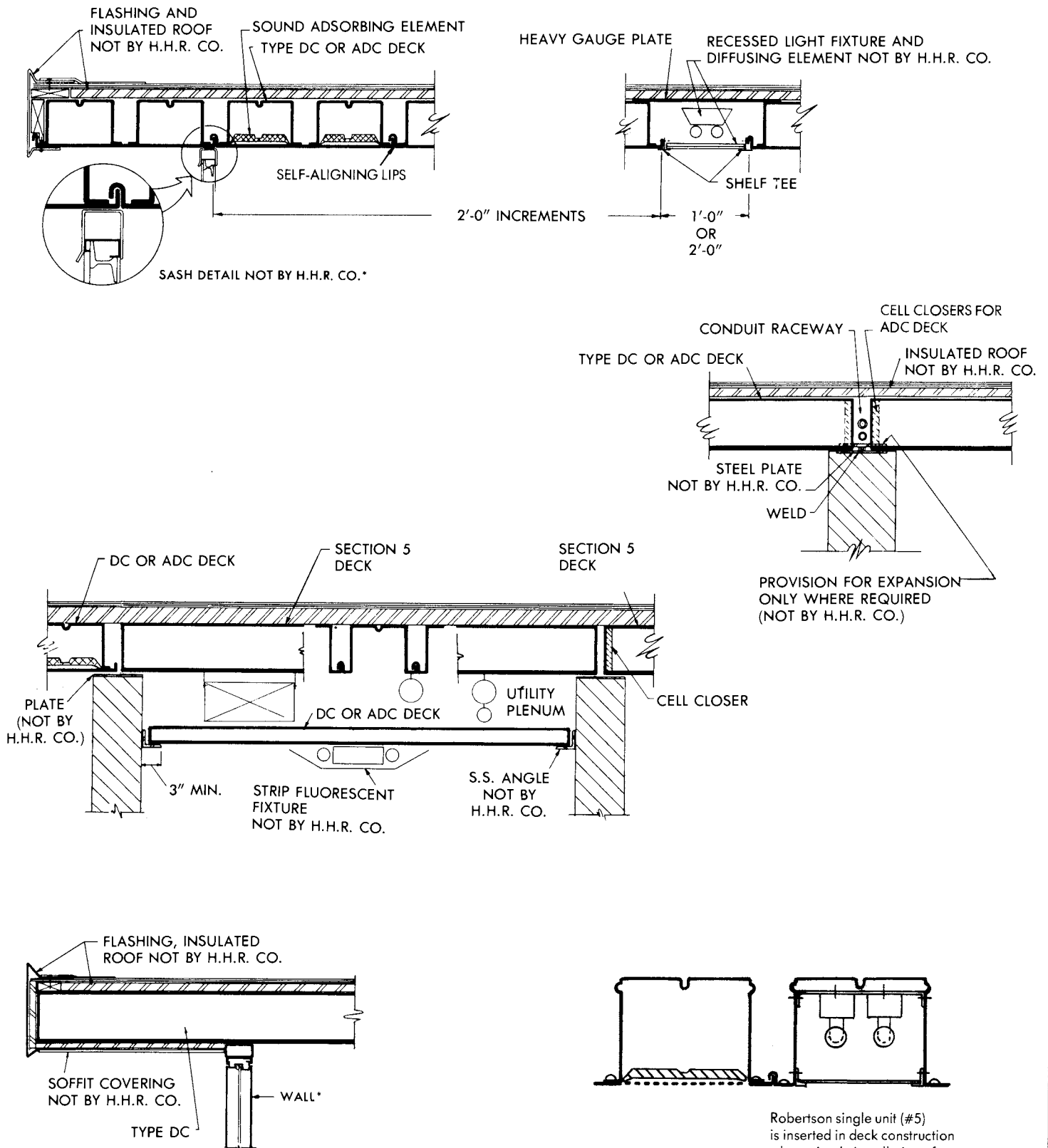


VALLEY DETAIL—ROOF



ROOF CELLS PARALLEL TO PARAPET WALL (MASONRY)

DC/ADC DECK



Robertson single unit (#5) is inserted in deck construction where simple installation of recessed lighting is desired.

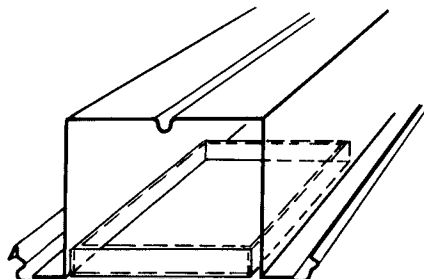
*Designer should consider deck deflection & its influence on this detail.



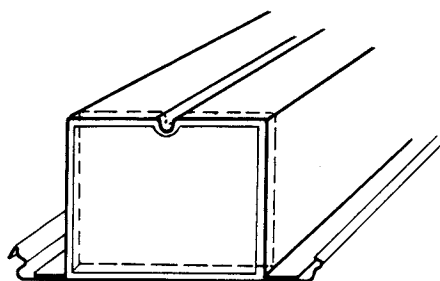
Robertson
Roofs

TYPICAL DETAILS - STRUCTURAL

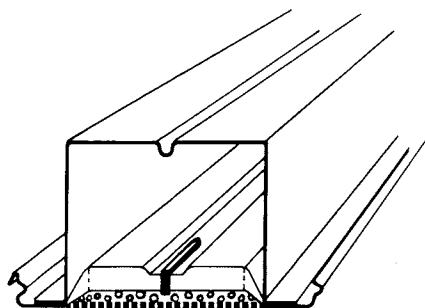
ADC AIR CEILING



PERFORATION-BLOCK

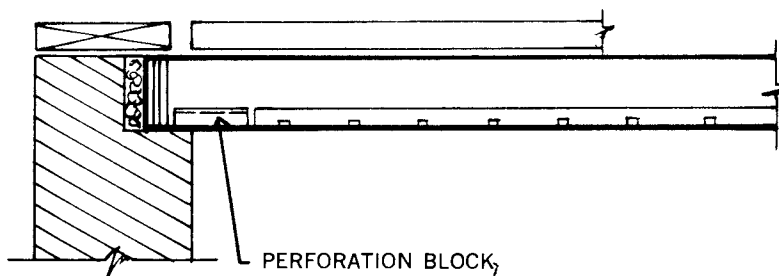


END CLOSURE



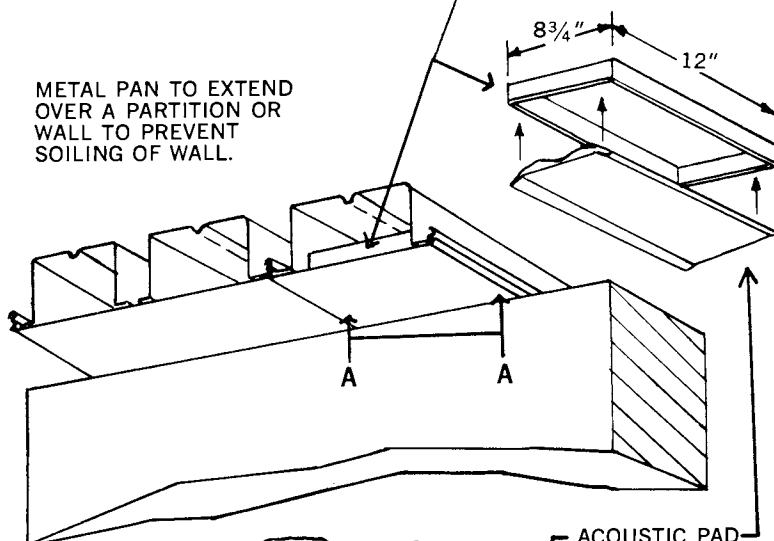
SECTION A-A

PAD AND PERFORATION-BLOCK



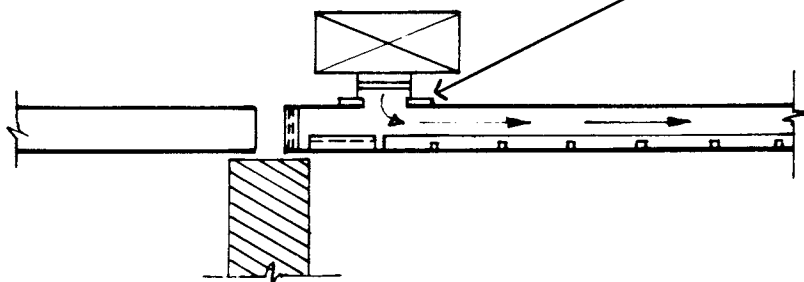
PERFORATION BLOCK

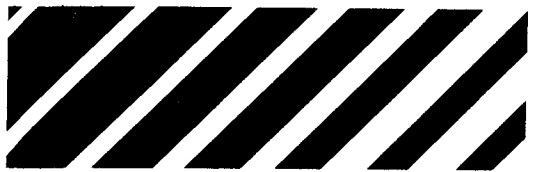
METAL PAN TO EXTEND
OVER A PARTITION OR
WALL TO PREVENT
SOILING OF WALL.



ACOUSTIC PAD-
PLACED IN THE
PERFORATION
BLOCK FOR
SOUND ABSORPTION

FIELD CUT REQUIRED OPENING
SECURE AND SEAL

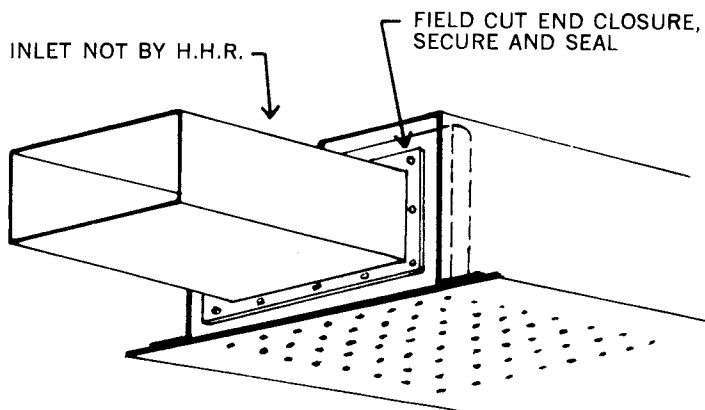
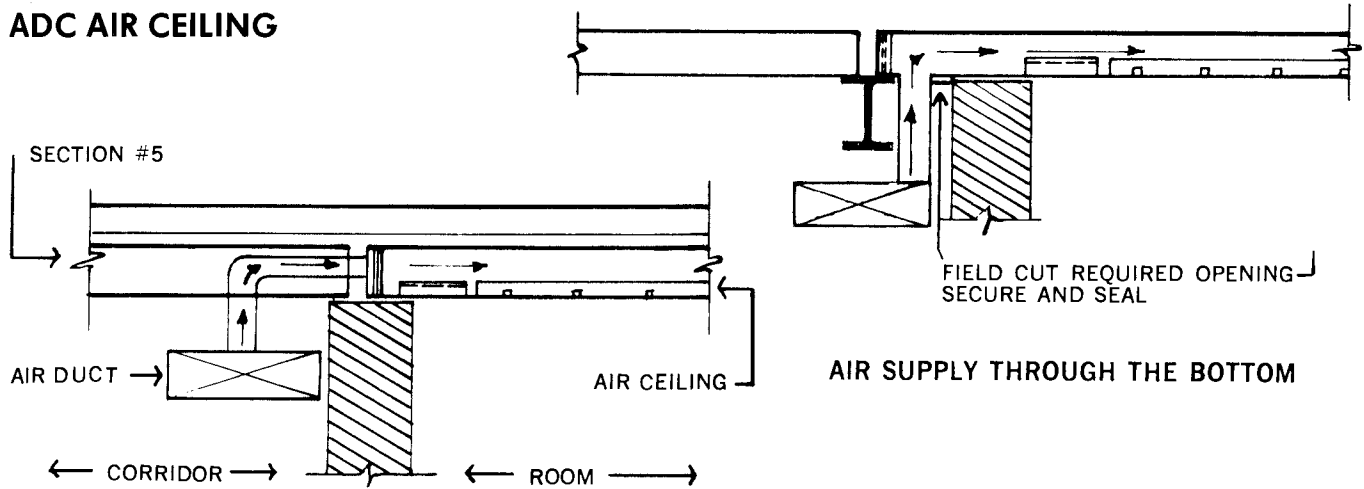




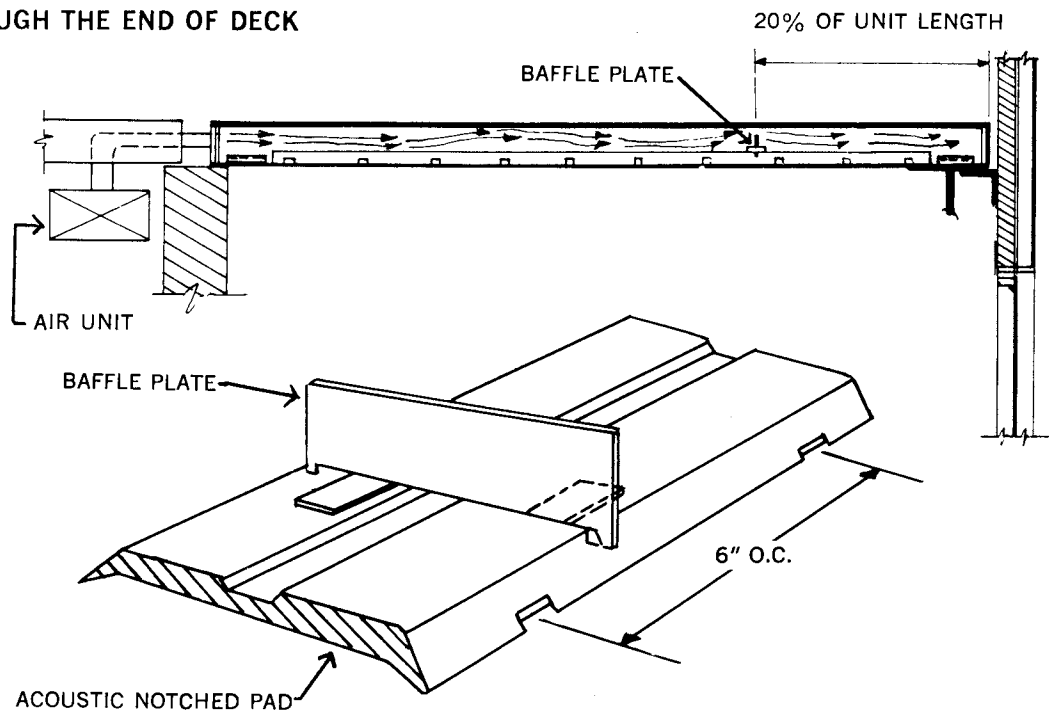
Robertson
Roofs

TYPICAL DETAILS - STRUCTURAL

ADC AIR CEILING



AIR SUPPLY THROUGH THE END OF DECK



BAFFLE PLATE



SPECIFICATIONS

UCI SECTION 05310 METAL ROOF DECK ROBERTSON SECTIONS 3, 21 and 12

PART 1—GENERAL

1.01 DESCRIPTION:

Provide all structural metal roof decking and sheet metal accessories specified in this section.

1.02 SHOP DRAWINGS:

The metal roof deck sub-contractor shall prepare and submit to the general contractor for approval by designer and contractor, erection/shop drawings which show the type of deck and gauge of steel being supplied, where it is to be located, necessary field fabrication, erection sequence and detail interface of deck with adjacent materials. These drawings shall also call or show all flashing which is to be supplied by the metal roof deck contractor. Drawings shall be submitted in _____ sets of prints for approval; after approval _____ sets shall be supplied for files and distribution.

PART 2—PRODUCTS

2.01 MATERIALS:

The steel deck and all flashing shall be formed from steel sheets, conforming to ASTM 446-76. The steel shall have received before being formed, a metal protective coating of zinc conforming to ASTM-A525-79 Class G-30 and to Federal Specifications QQ-S-775E.

2.02 DESIGN:

- a. The American Iron and Steel Institute's latest "Specification for the Design of cold formed steel Structural Members" shall govern the design of all roof deck units. The deck units shall be provided with an interlocking side lap. [Ends of deck units shall be countersunk to provide a smooth top surface at overlapping ends (except 14 and 12 ga.).] Roof deck units shall be in lengths to span over three or more supports wherever possible.
- b. Section #3 — Material shall be roll formed in sections 24" wide with custom cut lengths up to 40'. The units shall have four flutes 6" on center. The deck units shall be fabricated from (22—12 gauge, choose one) steel and be capable of supporting a live load of _____ psf, a dead load of _____ psf and maintaining a deflection of less than _____ (L/240 or L/360).
- c. Section #21 — Material shall be roll formed in sections 24" wide with custom cut lengths up to 40'. The units shall have three flutes 8" on center. The deck units shall be fabricated from (22—12 gauge, choose one) steel and be capable of supporting a live load of _____ psf, a dead load of _____ psf and maintaining a deflection of less than _____ (L/240 or L/360).

- d. Section #12 — Material shall be roll formed in sections 12" wide with custom cut lengths up to 40'. The units shall have two flutes 6" on center. The deck units shall be fabricated from (20 — 12 gauge, choose one) steel and be capable of supporting a live load of _____ psf, a dead load of _____ psf and maintaining a deflection of less than _____ (L/240 or L/360).

2.03 ACCESSORIES:

- a. Ridge and valley plates, metal cant strips and metal sump pans, which must be attached directly to the steel decks in order to provide a finished surface for the application of insulation and roofing, shall be furnished by the deck manufacturer.
- b. When decks rest on masonry walls or steel beams, over a partition or at an exterior wall, the hollow spaces between webs of the deck shall be closed with a pressed metal closure or die-cut neoprene filler where indicated on drawings. Closure or fillers shall be fastened to webs by means of sheet metal screws, welding or adhesive for neoprene fillers.

PART 3—EXECUTION

3.01 ERECTION

- a. Section 3 — Roof deck sheets shall be fastened to the supporting roof steel by 3/4" diameter fusion welds as follows:
 1. At outside edges of deck area, fasten in 1st, 3rd and 5th low corrugations. Fasten parallel edge 3' on center.
 2. At joints, fasten in 1st, 3rd and 5th low corrugations.
 3. At intermediate supports, fasten in 2nd and 4th low corrugations if span is greater than 4'-0".
 4. For spans of 4'-0" or less, fasten in one low cell at each intermediate support. Use the 2nd and 4th low cells alternately.
 5. Fasten side joints by:
 - a. Clinch side joints at 3'-0" centers for units 16 gauge and lighter.
 - b. One inch welds at 3'-0" on centers between supports for 12 and 14 gauge units.
- b. Section 21 — Roof deck sheets shall be fastened to the supporting roof steel by 3/4" diameter fusion welds as follows:
 1. At outside edges of deck area, fasten in 1st, 2nd and 4th low corrugations. Fasten parallel edge 3' on center.
 2. At joints, fasten in 1st, 2nd and 4th low corrugations.

(CONTINUED)



SPECIFICATIONS

ROBERTSON SECTIONS 3, 21 and 12

3. At intermediate supports, fasten in 1st and 3rd low corrugations.
4. Fasten side joints by:
 - a. Clinch side joints at 3'-0" centers for units 16 gauge and lighter.
 - b. One inch welds at 3'-0" on centers between supports for 12 and 14 gauge units.
- c. Erection for Section 12 Deck-Roof deck sheets shall be fastened to the supporting roof steel by 3/4" diameter fusion welds as follows:
 1. Fasten units at each support locating one weld along side the female lip and one at the middle low corrugation.
 2. Fasten side joints by:
 - a. Clinch side joints at 3'-0" maximum between centers for units 16 gauge and lighter.
 - b. One inch welds at 3'-0" on centers between supports for 12 and 14 gauge units.
- d. Cutting and flashing of openings other than framed openings shown on the structural drawings shall be framed, cut and flashed by others.
- e. Erection will comply with manufacturer's standards and with specific requirements of the approved metal roof deck shop drawings. All accessories specified of those drawings to be supplied by deck supplier will be erected by metal roof deck erector.
- f. Damage to steel deck profiles during the construction phase can significantly reduce their load carrying capacity. Deck erectors should exercise appropriate care to insure that any damaged deck is suitably strengthened or replaced.



SPECIFICATIONS

UCI SECTION 05310 METAL ROOF DECK ROBERTSON SECTIONS DC-ADC, AIR CEILING AND SECTION #5

PART 1—GENERAL

1.01 DESCRIPTION

Provide all structural metal roof decking and sheet metal accessories specified in this section.

1.02 SHOP DRAWINGS

The metal roof deck sub-contractor shall prepare and submit to the general contractor for approval by designer and contractor, erection/shop drawings which show the type of deck and gauge of steel being supplied, where it is to be located, necessary field fabrication, erection sequence and detail interface of deck with adjacent materials. These drawings shall also call or show all flashing which is to be supplied by the metal roof deck contractor. Drawings shall be submitted in _____ sets of prints for approval; after approval _____ sets shall be supplied for files and distribution.

PART 2—PRODUCTS

2.01 MATERIAL:

All of the deck units and associated flashing shall be fabricated from steel sheets conforming to ASTM-A446-76. The sheet shall have received a zinc coating conforming to ASTM-A525-79, Class A40.

2.02 DESIGN:

- a. The American Iron and Steel Institute's latest "Specification for the design of cold formed steel structural members" shall govern the design of all roof deck units.
- b. Type DC or ADC Roof Deck shall be composed of two identically formed beam sections with an integral stiffening rib rolled into the top flange of each section, and a flat plate. The flat plate shall have formed male and female self aligning side joints on opposite edges. These side joints shall be male-female type with continuous locking beads to insure positive vertical and lateral alignment of adjacent sections. The flat plate and the beam sections shall be assembled by electrical resistance spot welding to provide a structural cellular-beam unit. The flat plate for ADC Deck shall be perforated with 1/8" holes staggered 25/64" o.c. in two continuous paths 9" wide. The deck units shall be 24" in width and 1 1/2", 3", 4 1/2", 6" or 7 1/2" in depth, designated as 1.5, 3.0, 4.5, 6.0 and 7.5. The following nomenclature should be used — type — depth — hat gauge/plate gauge — example: ADC-7.5 — 18/16.
- c. Type "ADC Air Ceiling" deck shall be of identical construction as described for ADC deck except units shall be one single beam section mounted on a 12" width plate. Perforation pattern shall be such to permit the diffusion of air quantities as shown on mechanical drawing for specific areas. The type designation for these units shall be ADC "Air".

- d. Stucco embossed steel shall be used for the flat plate sections of all DC-ADC units. This shall be accomplished in embossing rolls, prior to forming the side laps, to remove rolling stresses, camber and to insure flatness of the sheet when assembled. Samples of the embossed pattern shall be approved by the architect prior to bid date, as part of a total deck assembly.
- e. After perforating and forming the side edges, the flat plate shall be degreased, steam cleaned and hot phosphate treated. The underside of the flat plate shall then receive a shop applied, oven cured prime coating compatible with standard field applied finish enamels.
- f. The sound absorbing elements in ADC deck shall be self-supporting, arch shaped, pressure and thermally-molded fiberglass pads which provide an air space of 1/2" between the perforated steel plate and pad. Metal chair supports are not permissible. The Noise Reduction Coefficient of the complete assembly shall be as determined by standard tests conducted by recognized acoustical laboratories.

The sound absorbing elements in the ADC Air Ceiling units shall be as described above, except shall be notched along edge supports to permit the flow of pressurized air from chamber above the pad to the perforated plate. Notch pattern shall be of such dimension to provide the air quantities specified or shown on mechanical drawings. The top side of all arched pads in the ADC Air Ceiling units shall be coated with neoprene, foil or other material to prevent air passage through the pad itself.

- g. Section #5 deck element shall consist of a single flute beam section 12" in width, with an integral stiffening rib rolled into the top flange. Bottom flanges shall have formed male and female side joints on opposite edges. Depth shall be 3", 4 1/2", 6" or 7 1/2", designated as 3.0, 4.5, 6.0 and 7.5. The following nomenclature should be used: Section #5 (depth) (gauge). Example: Section #5, 4.5-18.

2.03 ACCESSORIES:

- a. Ridge and valley plates and metal cant strips, and metal sump pans which must be attached directly to the steel decks in order to provide a finished surface for the application of insulation and roofing shall be furnished by the deck manufacturer.
- b. Cell end sound barrier closures of pressure and thermally-molded fiberglass shall be furnished by manufacturer wherever air-borne sound transmission can occur over walls and through deck cells at butt joint conditions. Closures shall be formed to cell profile to insure tight fit.
Where deck units are continuous over partition walls, sound barriers of 3"-thick, dimensionally oversize, laminar fiberglass shall be furnished for installation by the erector.
- c. Provide necessary sheet metal closure accessories for "ADC Air Ceiling." These accessories to be installed by the sheet metal or mechanical contractor.
- d. Provide glass fiber and metal closures, baffles and perforation blocks as shown on drawings to provide for air



SPECIFICATIONS

ROBERTSON SECTIONS DC-ADC, AIR CEILING AND SECTION #5

delivery through the ADC deck unit.

- e. Recessed lighting troffers shall be located and erected as indicated on the contract drawings.

DC or ADC units adjacent to recessed lighting troffer opening shall be increased to gauges indicated on drawings to support the additional load.

Shelf tees required to support diffusers for troffer lighting shall be furnished by the deck manufacturer and installed as specified under "Carpentry" section of specifications.

PART 3—EXECUTION

3.01 ERECTION:

Erection shall be by manufacturer or his qualified erector with proven experience and competence. Inaccuracies in alignment or level of bearing plates and structural supports shall be brought to the attention of proper parties and corrected by others before placement of deck. Proper bearing shall be provided at supporting members.

Units shall be attached to steel supports with three $\frac{1}{2}$ " fillet welds per unit end. All welding shall be electric arc welding performed by competent welders. Side joint welding, when necessary due to heavy gauge deck or diaphragm shear requirements, should be carefully made in the upper region of the side lap to minimize damage to underside coating below the weld area. All welds upon cooling shall be given a touch-up coat of paint top side only. Self-aligning side joints shall be integrated by button punching on 3' centers.

All holes or openings shall be cut and reinforced as shown in manufacturer's layout drawings.

3.02 WORK NOT INCLUDED:

- a. Painting — The touch-up of scuffs and abrasions due to transit or erection, the touch-up of charred points on the underside of the sidelap weld areas, as well as the field paint to the exposed surface of the galvanized steel deck shall be under the general painting specifications.

For those deck manufacturers who do not conform to the preparation and primer processes described by

para. 2.01 Material, this contractor shall clean down the exposed flat surfaces, field prime with a suitable primer for galvanized metal and then apply the specified number of finished coats.

- b. Steel Framing—All structural steel and structural steel attachments required to support the deck shall be furnished by the steel contractor. All steel framing shall be erected in conformance with tolerances set forth in AISC Standard Code of Practice.
- c. Built-up Roofing—Insulation—(24" Type DC (ADC) flat plate down)—Insulation shall be rigid type roof insulation _____ thick (minimum thickness and application based on roofer's standard recommendations) to span the 3" opening.
- d. Openings (not covered under Design)
Electrical—plumbing—heating—ventilating.
 - (1) All trades whose work involves the cutting of holes, reinforcing or drilling the deck shall furnish all work and labor necessary and at the cost of that trade whose work is affected. All such work shall be done in a neat, workman-like manner, without adversely affecting the structural value of the deck or finished appearance of exposed surfaces.
 - (2) At the option of the various sub-contractors whose work involves the cutting of holes, reinforcing or drilling the deck, the deck contractor shall furnish all work and labor necessary only during the time of his work at the job site, at the cost to the sub-contractor whose work is affected and at prices to be agreed upon by both parties.
- e. Electrical Work—The electrical contractor shall inspect all light troffer installations to assure that the opening width and height dimensions are acceptable for fixture installation and diffuser attachment.
- f. Mechanical Work—When the ADC Air Ceiling units are to be used for air conditioning the mechanical contractor shall include all necessary cutting preparation and attachment of ducts to air cells and coordinate them with the AC system.

Photo Credits

Page Three

1. St. Paul Civic Center, St. Paul, Minnesota
2. Idaho State University Field House, Pocatello, Idaho
3. Essex Community College Field House, Essex, Maryland
4. Mississippi Coasts Coliseum, Biloxi, Mississippi
5. Notre Dame University Field House, South Bend, Indiana

Page Four

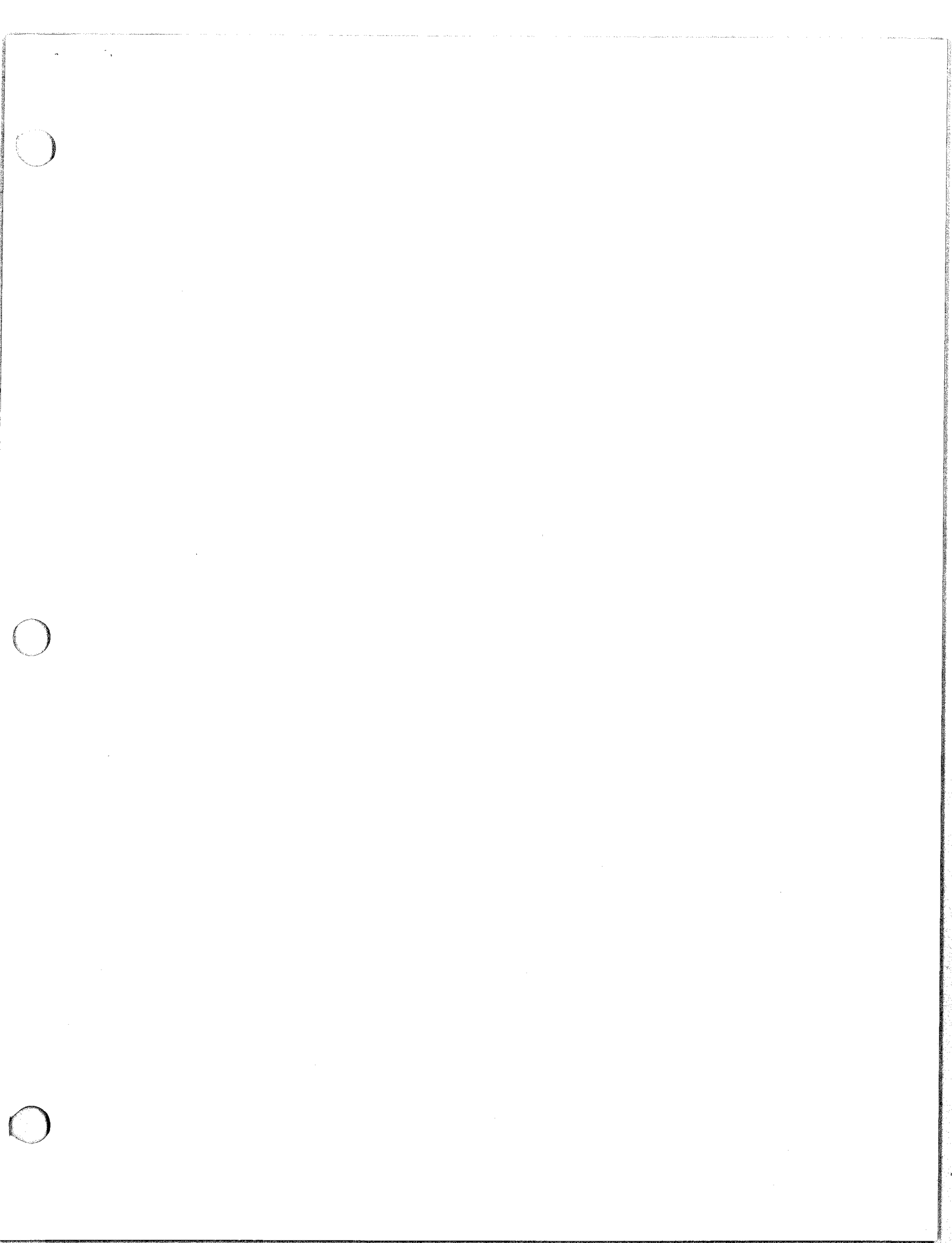
6. Southern Illinois University, Carbondale, Illinois
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8. University of Michigan School of Architecture, Ann Arbor, Michigan
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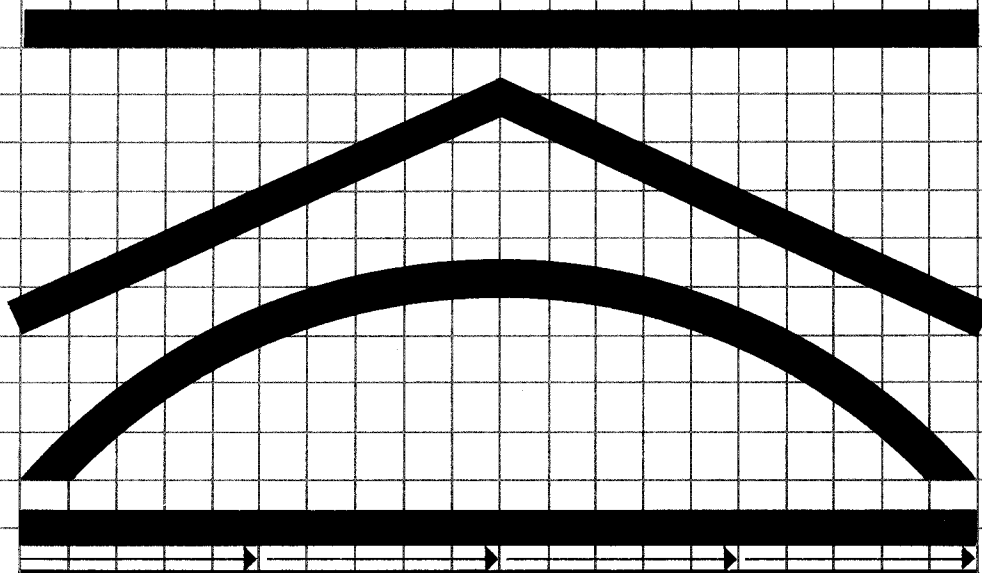
Page Five

10. Fayette County Correctional Facility, Lexington, Kentucky
11. Bay County Jail, Panama City, Florida
12. Baldwin County Correctional Facility, Milledgeville, Georgia
13. Parish Prison, New Orleans, Louisiana

Page Six

14. Logan International Airport, Boston, Massachusetts
15. Baxter-Travenol Laboratories, Chicago, Illinois
16. Sim Gideon Steam Plant, Bastrop, Texas
17. United Air Lines Baggage Claim Area, San Francisco, California





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Printed in U.S.A.
Issued January, 1975
Reprinted March, 1981
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Q-182-81 (SL)